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# **A CONJUNCTIVE USE SURFACE WATER-GROUND WATER SIMULATOR**

Partial Technical Completion Report  
Project No. A-045-NMEX

A CONJUNCTIVE USE SURFACE WATER-GROUND WATER SIMULATOR

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Project No. A-045-NMEX

New Mexico Water Resources Research Institute  
*in cooperation with*  
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## ABSTRACT

As part of an interdisciplinary analysis of the economic evaluation of the water resources of the Rio Grande Region of New Mexico, a conjunctive use surface water-ground water simulator was developed. The simulator was used to determine the dynamic availability of ground water and its relationship to flow in the Rio Grande.

This report gives an account of the structure and use of the simulator. It is a computer model in which the governing nonlinear parabolic partial differential equation, used to simulate unconfined aquifer flow in two dimensions, was solved by implicit finite differences. The set of difference equations in residual form are solved by LSOR or Gauss Elimination depending upon the size of the problem.

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# A CONJUNCTIVE USE SURFACE WATER-GROUND WATER SIMULATOR

by W. Brutsaert and C. Way\*

## I. GENERAL INTRODUCTION

The complexity of today's water resource systems makes it almost mandatory that their management be achieved through the application of the talents and technology of a variety of disciplines, and the legal and institutional structures of water use make their understanding and application a further requisite to good management. Although economic justification should be the foundation of decisions on alternative uses of water, the social and cultural implications must be fully considered since the optimal use of water implies a maximization of the benefits returned to society through a broad range of beneficial uses. In order to formulate plans and policies for future water resources development, the assessment of future water supplies and requirements necessitates a major consideration of future rates and patterns of economic development.

Before an economic evaluation of water use can be made, the availability of surface water and ground water must be determined. If there is a continual exchange between the surface water of the river and the ground water of the surrounding alluvium, ground water availabilities are not simply related to pumpage but are also controlled by precipitation, amount and frequency of runoff in streams, return of irrigation water, and evapotranspiration. The management of such a system should consequently be based on a conjunctive operation of surface and ground water.

A model of the exchange of ground water and surface water is necessary if a good description of the pumping effects is desired. In addition, the diversions and depletions must be known in order to have a reliable model.

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For a comprehensive alternative water use analysis it is necessary to know both the ground water availability and the behavior of the aquifer under projected stresses. Since historical records of hydrologic systems, in most instances, are inadequate to permit direct analysis of basin behavior under projected stresses, a ground water system simulator is necessary. The most efficient and practical simulator appears to be a mathematical analogue of the hydrologic basin, solved by digital computer. Such a hydrologic system can be schematically represented as in Figure 1:1.

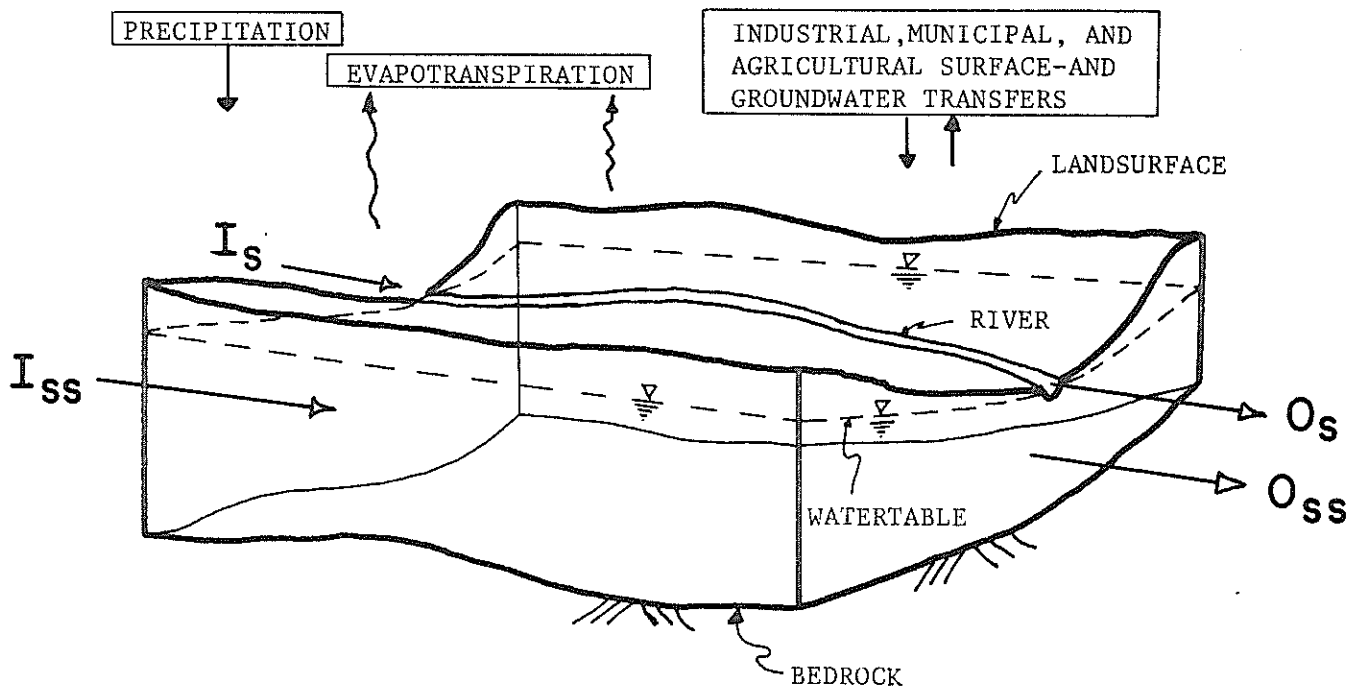


Figure 1:1. Schematic representation of hydrologic system.

In this Figure, I = inflow, O = outflow, and the subscripts s and ss, respectively, stand for surface and subsurface. Figure 1:1 is simply a representation of the statement of continuity: i.e., the conservation of

mass which is the basis for developing the fundamental flow equation or mathematical analogue which is written as:

$$\frac{\partial}{\partial x} (Kh \frac{\partial H}{\partial x}) + \frac{\partial}{\partial y} (Kh \frac{\partial H}{\partial y}) = S \frac{\partial H}{\partial t} + \frac{Q}{\delta x \delta y} \quad (1:1)$$

where K = hydraulic conductivity; h = saturated thickness;  
H = water table elevation (total head) above datum;  
S = effective porosity (storage coefficient); Q = net  
withdrawal rate from aquifer; x, y = space dimensions;  
t = time dimensions;  $\delta x$ ,  $\delta y$  = surface area of element.

The above equation is a nonlinear partial differential equation obtained by combining Darcy's law with the continuity principle and is applicable to transient, two-dimensional flow in heterogeneous, anisotropic, incompressible, unconfined, saturated, porous media.

Implicit finite differences are utilized to solve equation 1:1 on a digital computer. The resulting system of simultaneous equations is either solved directly by Gauss-Elimination or by Line Successive Over Relaxation, depending upon the size of the problem: i.e., number of equations.

The first step when applying the model to a particular study area is to verify or calibrate its behavior. This verification consists of simulating a historical period, for which both ground water levels and stream records are available, until model and prototype match. Checking and updating should be continued as more data become available.

The next step is the simulation (or extrapolation) of future possible conditions. Conditions can be altered to give different responses. Conditions from extreme dry to extreme wet combined with a set of water demands ranging from large to small may be included.

A vast amount of data is thus obtained in the form of aquifer responses for given conditions. Realizing that these data are the result of the solution of a continuity equation of the form

$$I - O = \Delta S / \Delta t \quad (1:2)$$

it is possible to estimate an analogous relationship from the data obtained. In equation 1:2, I = inflow, O = outflow, and  $\Delta S$  = change in storage during a time period  $\Delta t$ . The relationship postulated was of the following form

$$\Delta d = f(d_n, L) \quad (1:3)$$

where  $\Delta d$  = change in water-table elevation for the time period (year) considered,  $d_n$  = water elevation at the end of previous time period (year), and  $L$  = a lump factor combining surface water inflow and outflow, precipitation, and beneficial and nonbeneficial water uses.

Results of the simulation runs can be tabulated with averaged spatial responses: i.e., results would not reflect a variation in water-table elevation along lines perpendicular to the river bed. It is assumed that average conditions would suffice since lateral aquifer response would average out.

A stepwise multiple regression analysis, combining linear and nonlinear (exponential and logarithmic) terms of the different factors obtained from the simulation runs, can be performed to obtain the surface-ground water interrelationship equation.

This conjunctive surface-ground water model was developed and applied to the Rio Grande region in New Mexico. A presentation of the results from this application is given in WRRRI reports 020-024 (Lansford et al., 1973).

## II. TECHNICAL MODEL DESCRIPTION

Adequate simulation of complex ground water systems can be accomplished using digital computer solutions of a mathematical model. This model was developed by using a general mass balance equation and then calibrated by simulating historical conditions. If computed and actual water-table elevations agree within some preset tolerance, then the mathematical model provides a means of predicting the effects of future ground water development.

Ideal boundary conditions and water-bearing characteristics are rarely if ever found in nature, and therefore analytical approaches, when applied to ground water systems, become oversimplified. Most aquifers are heterogeneous and anisotropic, with irregular boundaries. Mathematical models are ideal for handling such natural systems. However, hydrogeologic judgement should be utilized in designing these mathematical models to approximate actual conditions and in properly qualifying the results according to the amount of discrepancy.

Selection of the space and time dimensions ( $\Delta X$ ,  $\Delta Y$ ,  $\Delta t$ ) is dependent upon the availability of geologic and hydrologic data and upon the desired accuracy and detail of analysis. The accuracy of the solution is enhanced as the dimensions of  $\Delta X$ ,  $\Delta Y$ , and  $\Delta t$  are decreased, providing the availability of geologic and hydrologic data justify the additional computational time required. Space dimensions should be small enough so that the geologic and hydrologic conditions may be assumed reasonably uniform over an entire grid. The maximum size of the time increment,  $\Delta t$ , which will provide adequate accuracy should be used. Solutions of sample problems with representative data can be used to determine the optimum value of  $\Delta t$  for selected grid dimensions. Smaller grid dimensions may require shorter time increments.

A variable  $\Delta X, \Delta Y$ -grid system was utilized in this model in order to adjust the size of the grid blocks to the required detail of analysis.

#### Continuous and Discretized Mass Balance Equation

The non-linear partial differential equation describing transient, incompressible, isothermal, two-dimensional flow in an unconfined saturated porous medium may be derived from the mass continuity equation and Darcy's Law and can be written as:

$$\frac{\partial}{\partial x}(K_{xx} h \delta y \frac{\partial H}{\partial x}) \delta x + \frac{\partial}{\partial y}(K_{yy} h \delta x \frac{\partial H}{\partial y}) \delta y = Q + \delta x \delta y S \frac{\partial H}{\partial t} \quad (2:1)$$

where,

- $K_{xx}$  = principal component of hydraulic conductivity tensor (coinciding with x-direction). [L/T]
- $K_{yy}$  = principal component of hydraulic conductivity tensor (coinciding with y-direction). [L/T]
- $h$  = saturated thickness of aquifer. [L]
- $S$  = storage coefficient. (dimensionless)
- $Q$  = net rate of withdrawal. [ $L^3/T$ ]
- $\delta x, \delta y$  = dimensions of differential element.
- $x, y$  = space dimensions. [L]
- $t$  = time dimension. [T]
- $H$  = water table elevation above a datum (potentiometric head). [L]

This equation is valid for heterogeneous and anisotropic media but is subject to the Dupuit-Forchheimer assumptions and should be used with these limitations in mind. The most important limitations are flow in the horizontal x,y-plane, and fully penetrating constant head (river, lake) boundary conditions.

Equation 2:1 has no general analytical solution; therefore, a finite difference approximation is utilized to obtain a numerical solution with the aid of a digital computer. Application of the finite difference approach requires subdivision of the study area into a system of finite grids.

Consider the following two blocks (Figure 2:1) of this grid system:

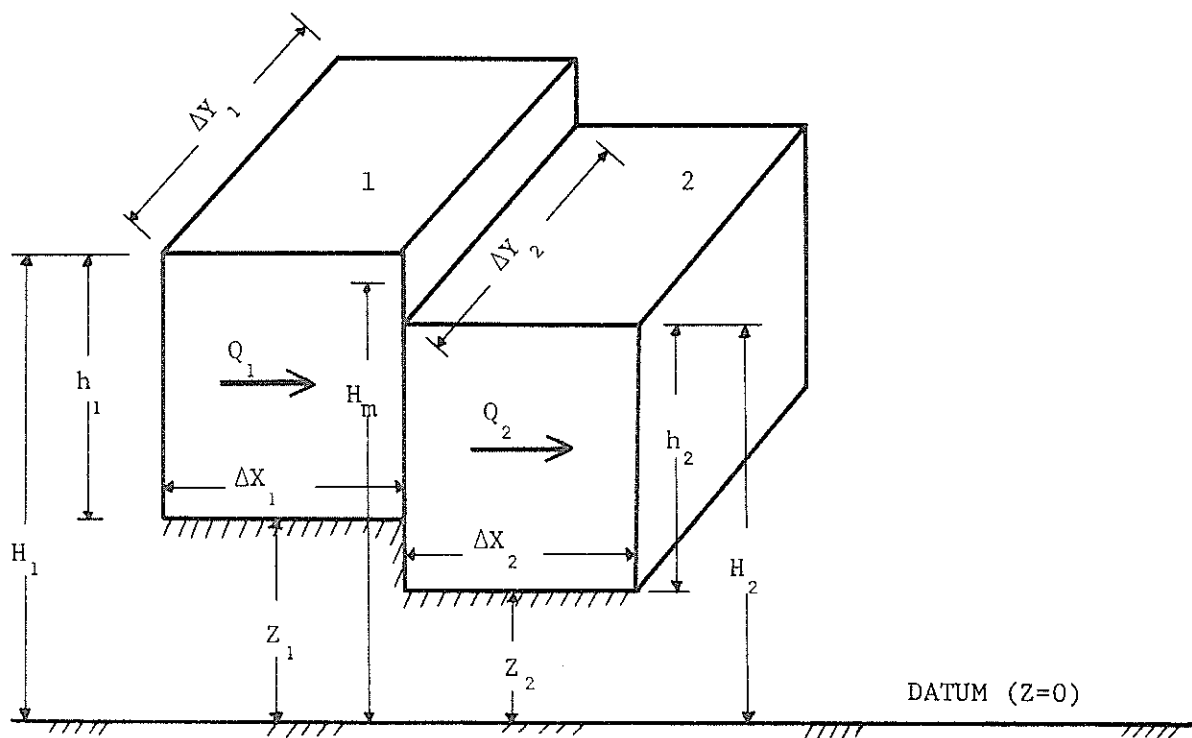


Figure 2:1. Idealization of two adjacent blocks.

According to Darcy's Law:

$$Q_1 = K_1 h_1 \Delta Y_1 \frac{H_m - H_1}{\Delta X_1 / 2} \quad (2:2)$$

$$Q_2 = K_2 h_2 \Delta Y_2 \frac{H_2 - H_m}{\Delta X_2 / 2} \quad (2:3)$$

where

$H_m$  = potentiometric head at the interface of block 1 and 2. [L]

$Z$  = bedrock elevation above an arbitrary datum. [L]

Since, by continuity,  $Q_1 = Q_2 = Q$ ,  $H_m$  in equations 2:2 and 2:3 can be eliminated to obtain the following relation:

$$Q = \frac{2(K_1 h_1 \Delta Y_1) (K_2 h_2 \Delta Y_2)}{K_2 h_2 \Delta Y_2 \Delta X_1 + K_1 h_1 \Delta Y_1 \Delta X_2} (H_2 - H_1) \quad (2:4)$$

this expression can be approximated by:

$$Q = \frac{2K_1 \Delta Y_1 K_2 \Delta Y_2 \bar{h}_{12}}{K_1 \Delta Y_1 \Delta X_2 + K_2 \Delta Y_2 \Delta X_1} (H_2 - H_1) \quad (2:5)$$

where,

$$\bar{h}_{12} = \text{MAX} (H_2, H_1) - \text{MAX} (Z_2, Z_1) \quad (2:6)$$

MAX (A,B) means the largest of the two quantities A and B.

By using this technique for computing flow coefficients and an implicit central finite difference form, equation 2:1 becomes:

$$\begin{aligned} & C_E H_{i+1,j}^{t+\Delta t} + C_W H_{i-1,j}^{t+\Delta t} + C_S H_{i,j+1}^{t+\Delta t} + C_N H_{i,j-1}^{t+\Delta t} \\ & - (C_E + C_W + C_N + C_S + \frac{S \Delta X \Delta Y}{\Delta t}) H_{i,j}^{t+\Delta t} \\ = & - (\frac{S \Delta X \Delta Y}{\Delta t}) H_{i,j}^t + Q_{i,j} \end{aligned} \quad (2:7)$$

$C_E, C_W, C_N, C_S$  are the flow coefficients of the mass balance equation. Subscripts E, W, N, S denote the directions east, west, north, and south: they are used for convenience to replace the more standard but lengthy notation of  $i+\frac{1}{2}, j, i-\frac{1}{2}, j, i, j-\frac{1}{2}, i, j+\frac{1}{2}$ , and they reflect properties of two adjacent gridblocks:

$$C_W = \frac{2K_{i-1,j} K_{i,j} \Delta Y_{i-1,j} \Delta Y_{i,j} \bar{h}_W^t}{\Delta X_{i,j} K_{i-1,j} \Delta Y_{i-1,j} + \Delta X_{i-1,j} K_{i,j} \Delta Y_{i,j}} \quad (2:8)$$

$$C_E = \frac{2K_{i,j}K_{i+1,j}\Delta Y_{i,j}\Delta Y_{i+1,j}\bar{h}^t}{\Delta X_{i+1,j}K_{i,j}\Delta Y_{i,j} + \Delta X_{i,j}K_{i+1,j}\Delta Y_{i+1,j}}$$

$$C_N = \frac{2K_{i,j-1}K_{i,j}\Delta X_{i,j-1}\Delta X_{i,j}\bar{h}^t}{\Delta Y_{i,j}K_{i,j-1}\Delta X_{i,j-1} + \Delta Y_{i,j-1}K_{i,j}\Delta X_{i,j}}$$

(2:8)  
Cont'd.

$$C_S = \frac{2K_{i,j+1}K_{i,j}\Delta X_{i,j+1}\Delta X_{i,j}\bar{h}^t}{\Delta Y_{i,j}K_{i,j+1}\Delta X_{i,j+1} + \Delta Y_{i,j+1}K_{i,j}\Delta X_{i,j}}$$

The  $i,j$  notation (Figure 2:2) refers to the grid block at which a particular equation is formulated, and the superscripts represent the time level of computation.

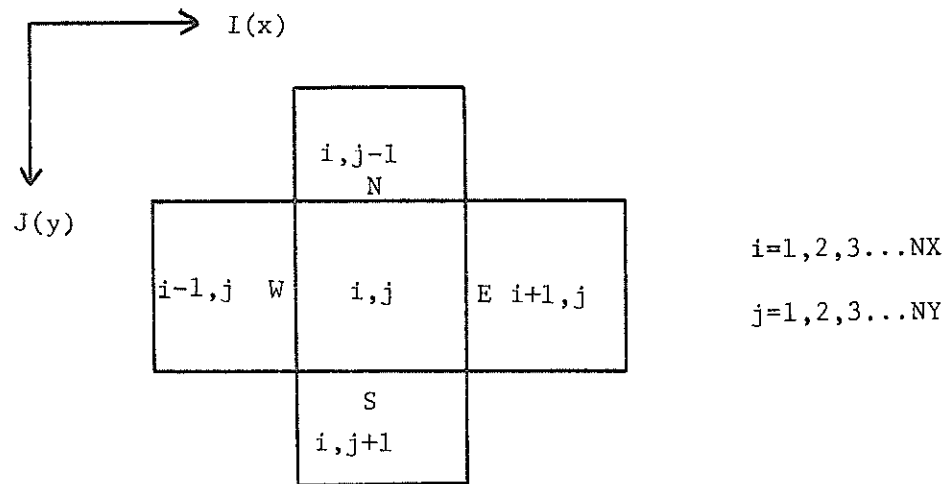


Figure 2:2. Numbering system for  $i,j$  notation.

In Figure 2:2,  $I$  and  $J$  denote block numbers in the coordinate system  $(X,Y)$ , and  $i$  and  $j$  are subscripts used to identify grid blocks.  $NX$  and  $NY$  are numbers of blocks in  $x$  and  $y$  directions.

Let

$$C_E + C_W + C_N + C_S + \frac{S\Delta X\Delta Y}{\Delta t} = A$$

$$-\left(\frac{S\Delta X\Delta Y}{\Delta t}\right) H_{i,j}^t + Q_{i,j} = -B$$

(2:9)

Equation 2:7 then becomes:

$$A_{i,j} H_{i,j}^{t+\Delta t} - C_E H_{i+1,j}^{t+\Delta t} - C_W H_{i-1,j}^{t+\Delta t} - C_N H_{i,j-1}^{t+\Delta t} - C_S H_{i,j+1}^{t+\Delta t} = B_{i,j} \quad (2:10)$$

To save computer time, single instead of double dimensioned arrays were used. To understand the relationship between single (ij) and double (i,j) subscripted variables, compare Figure 2:2 with Figure 2:3. Equation 2:10 written with ij notation (Figure 2:3) becomes:

$$A_{ij} H_{ij}^{t+\Delta t} - C_E H_{ij+1}^{t+\Delta t} - C_W H_{ij-1}^{t+\Delta t} - C_N H_{ij-NX}^{t+\Delta t} - C_S H_{ij+NX}^{t+\Delta t} = B_{ij} \quad (2:11)$$

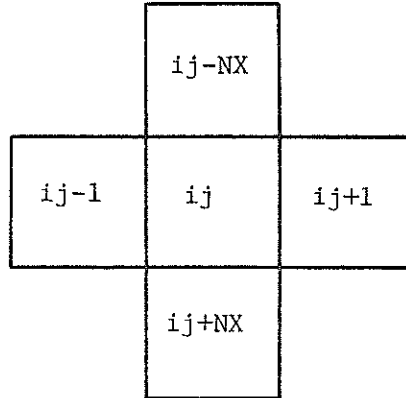


Figure 2:3. Numbering system for ij notation.

It should be noted that the coefficients  $C_E$ ,  $C_W$ ,  $C_N$ ,  $C_S$  and the net rate of withdrawal  $Q$  are held constant during each time step.

Equation 2:11 is written for each grid for a particular time step and the entire system of equations thus obtained is solved simultaneously. The solution will be the predicted value of  $H$ , the water-table elevation, at the end of the time step. This predicted value will be used as the initial value for the next time step, and the entire process is repeated.



### III. METHODS OF SOLUTION

Residual Approach. The residual approach as used in this program is simply an aid to improve accuracy through single precision calculations for the solution of systems of equations. IBM SYSTEM 360, MOD 44, with its 6 to 7 digit accuracy, makes this approach obligatory.

Equation 2:11 can be expressed in matrix notation as follows:

$$[D] [H]^{t+\Delta t} = [RHS] \quad (3:1)$$

where

[D] = the coefficient matrix

[H]<sup>t+Δt</sup> = water elevation vector at time t+Δt (variable)

[RHS] = right-hand side constant vector

Define

$$H^{t+\Delta t} = H^k + H^* \quad (3:2)$$

where

H<sup>k</sup> = the computed value for k-th iteration

H<sup>\*</sup> = the correction between the computed value and the actual result.

In matrix form equation 3:2 can be written as:

$$[H]^{t+\Delta t} = [H]^k + [H]^* \quad (3:3)$$

[H]<sup>\*</sup> is then called the correction vector. Substituting 3:3 into 3:1 produces:

$$[D] [H]^* = [r] \quad (3:4)$$

where [r] = [RHS] - [D] [H]<sup>k</sup>, and is called residual vector.

Instead of equation 3:1, equation 3:4 is solved. The standard procedure is as follows:

- (1) Calculate the residual vector [r] in double precision (using local variables).
- (2) Solve for the correction vector [H]<sup>\*</sup> in single precision.
- (3) Solve for [H]<sup>t+Δt</sup> = [H]<sup>k</sup> + [H]<sup>\*</sup> in double precision (using local variables).

If the desired accuracy is not obtained, new residuals are computed and the entire process is repeated until equation 3:1 is satisfied (i.e., until residuals have vanished).

The obvious advantage of the residual approach is that a solution can be made accurate beyond the normal number of single precision digits, and extreme accuracy can be obtained when required.

### Gaussian Elimination

Gaussian Elimination is a direct method for solving sets of linear algebraic equations. In general, this method tends to be more effective than iterative methods when:

- (1) the matrix of coefficients is too complex for a rapid estimate of the optimum over-relaxation factor, or
- (2) the matrix is almost singular in that small residuals do not imply small errors in the solution, or
- (3) several sets of equations with the same coefficients but different constants have to be solved, or
- (4) one-dimensional or two-dimensional problems with relatively small number of grids are solved.

The disadvantages of Gaussian Elimination are that round-off error may affect accuracy seriously; that it is time consuming for two dimensional problems; and that storage requirements increase almost exponentially with the size of the grid system.

In a two-dimensional problem each equation (2:11) consists of five unknowns ( $H_{ij}$ ,  $H_{ij-1}$ ,  $H_{ij+1}$ ,  $H_{ij-NX}$ ,  $H_{ij+NX}$ ) (see Fig. 2:3). In a block system with  $m$  rows and  $n$  columns, there will be  $m \times n$  equations with  $m \times n$  unknowns. Therefore, in equation 3:1,  $[D]$  is a coefficient matrix with the size of  $m \times n$  by  $m \times n$ .

As an example, for a 3 x 3 block system (Fig. 3:1),  $m = NX = 3$ ,  
 $n = NY = 3$ .

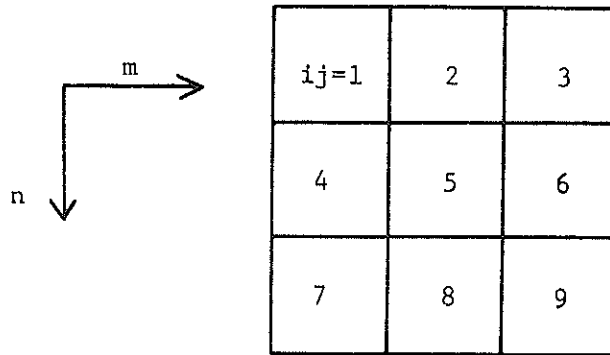


Figure 3:1. Simple 3 x 3 grid system.

For this special case, the coefficient matrix is a square matrix with the size 9 x 9. (Fig. 3:2).

		← $2m+1=7$ →										
		1	2	3	4	5	6	7	8	9		
1	A <sup>1</sup>	C <sub>E</sub> <sup>1</sup>	0	C <sub>S</sub> <sup>1</sup>	0	0	0	0	0	0	1	
2	C <sub>W</sub> <sup>2</sup>	A <sup>2</sup>	C <sub>E</sub> <sup>2</sup>	0	C <sub>S</sub> <sup>2</sup>	0	0	0	0	0	2	
3	0	C <sub>W</sub> <sup>3</sup>	A <sup>3</sup>	C <sub>E</sub> <sup>3</sup>	0	C <sub>S</sub> <sup>3</sup>	0	0	0	0	3	
4	C <sub>N</sub> <sup>4</sup>	0	C <sub>W</sub> <sup>4</sup>	A <sup>4</sup>	C <sub>E</sub> <sup>4</sup>	0	C <sub>S</sub> <sup>4</sup>	0	0	0	4	
5	0	C <sub>N</sub> <sup>5</sup>	0	C <sub>W</sub> <sup>5</sup>	A <sup>5</sup>	C <sub>E</sub> <sup>5</sup>	0	C <sub>S</sub> <sup>5</sup>	0	5		
6	0	0	C <sub>N</sub> <sup>6</sup>	0	C <sub>W</sub> <sup>6</sup>	A <sup>6</sup>	C <sub>E</sub> <sup>6</sup>	0	C <sub>S</sub> <sup>6</sup>	6		
7	0	0	0	C <sub>N</sub> <sup>7</sup>	0	C <sub>W</sub> <sup>7</sup>	A <sup>7</sup>	C <sub>E</sub> <sup>7</sup>	0	7		
8	0	0	0	0	C <sub>N</sub> <sup>8</sup>	0	C <sub>W</sub> <sup>8</sup>	A <sup>8</sup>	C <sub>E</sub> <sup>8</sup>	8		
9	0	0	0	0	0	C <sub>N</sub> <sup>9</sup>	0	C <sub>W</sub> <sup>9</sup>	A <sup>9</sup>	9		
		1	2	3	4	5	6	7	8	9		

Figure 3:2. Coefficient matrix [D].

In Figure 3:2, superscripts denote the block numbers, subscripts denote the directions. The coefficient matrix has values only on five diagonals: i.e., the main diagonal, two adjacent diagonals, and two outer diagonals.

The distance (in terms of matrix columns) between the outer diagonals is the distance between  $C_N$  and  $C_S$ . This can be calculated by using a simple relation: i.e., distance =  $2m + 1$ . In this case it is  $2 \times 3 + 1 = 7$ .

This matrix is a sparse five diagonal matrix, and it is obvious that much storage would be wasted if all zero values were stored in computer memory. How to transform this matrix to a more condensed form in order to save storage is the basis for "band algorithm" (Rosenberg, 1969; Thurnau, 1963). This technique rotates the matrix to the form shown in Figure 3:3. The size of this new matrix is  $m \times n$  by  $2m + 1$ .

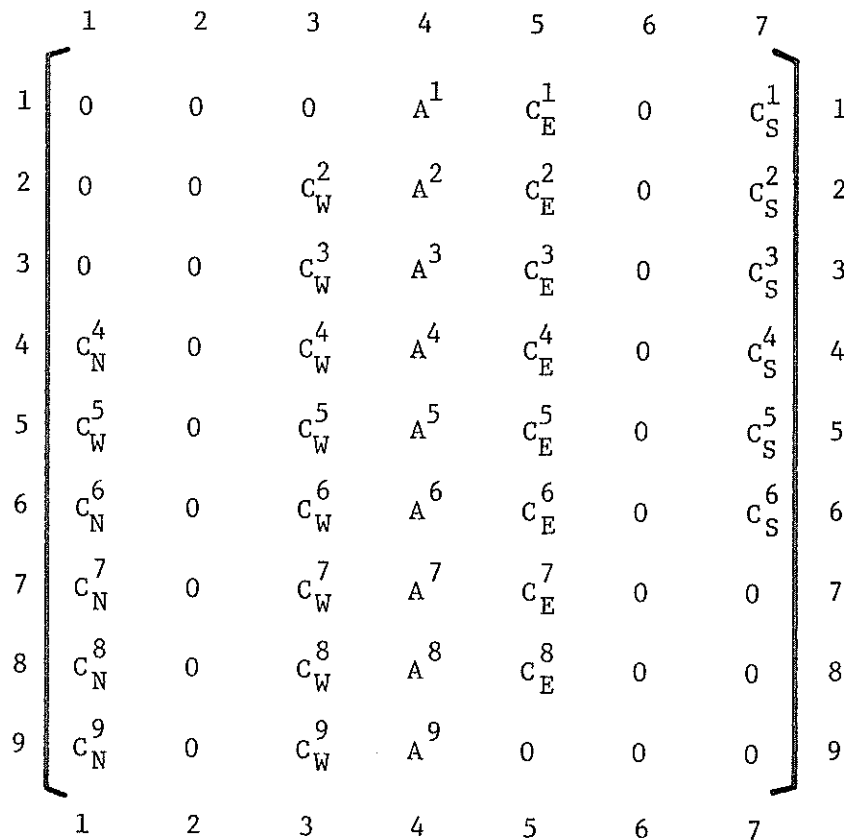


Figure 3:3. Band matrix.

All coefficients are now stored in a  $[m \times n]$  by  $[2m + 1]$  matrix rather than a  $[m \times n]$  by  $[m \times n]$  matrix. In this particular case a 9 by 7 matrix does not seem to be much smaller than a 9 by 9 matrix. However, consider for example  $m = n = 20$ : a 400 by 41 matrix and a 400 by 400 matrix are considerably different. For grid systems in which the number of rows is not equal

to the number of columns ( $NX \neq NY$ ), it is important to organize the grid system such that  $NX$  is less than  $NY$ .

Line Successive Over Relaxation Method (LSOR)

The algorithm for point successive over relaxation method (SOR) in residual form is (see Figure 3:4):

$$H_{ij}^{*k+1} = (1 - \omega)H_{ij}^{*k} + \frac{\omega}{A} \left( C_N H_{ij-NX}^{*k+1} + C_W H_{ij-1}^{*k+1} + C_E H_{ij+1}^{*k} + C_S H_{ij+NX}^{*k} - r_{ij} \right) \quad (3:5)$$

where superscripts  $k$  and  $k + 1$  represent  $k^{\text{th}}$  and  $k + 1^{\text{th}}$  iterations, and  $\omega$  is the over-relaxation parameter omega with a value between 1 and 2.

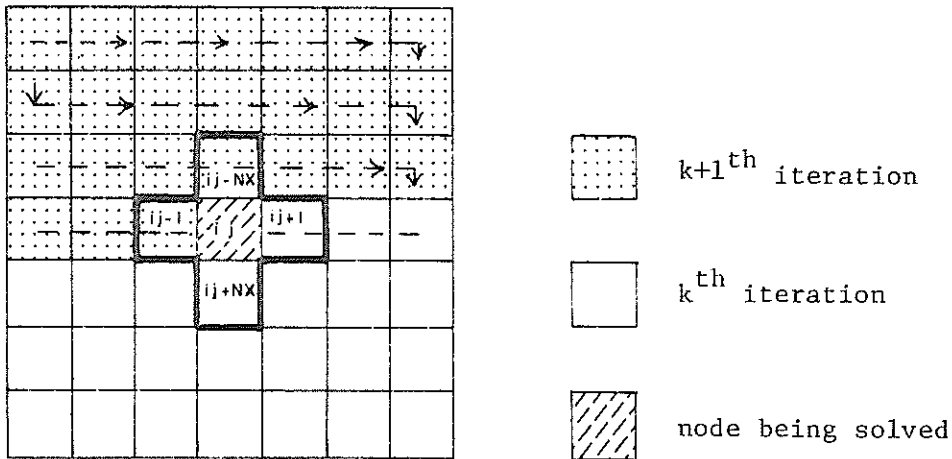


Figure 3:4. SOR method.

The only unknown in equation 3:5 is  $H_{ij}^{*k+1}$ . Consideration of equation 3:5 and Figure 3:4 shows that in order to solve for  $H_{ij}^{*k+1}$ , the old  $k$ -th iteration values ahead ( $H_{ij}^{*k}$ ,  $H_{ij+1}^{*k}$ ,  $H_{ij+NX}^{*k}$ ) and the new  $(k + 1)$ -th iteration values behind ( $H_{ij-NX}^{*k+1}$ ,  $H_{ij-1}^{*k+1}$ ) are used; hence, only one matrix

value of  $[H]^*$  needs to be stored. Equation 3:5 is set up and solved point by point until the whole system is completed. This procedure is called point successive over relaxation (SOR).

Instead of solving an equation point by point, the SOR algorithm can be modified to solve the problem row by row (or column by column): this process is called line successive over relaxation method (LSOR). (Figure 3:5.)

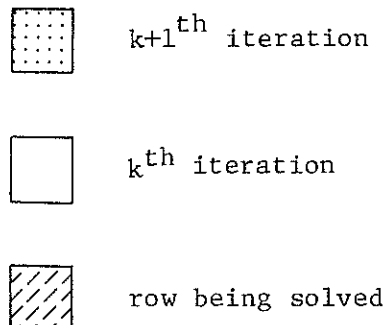
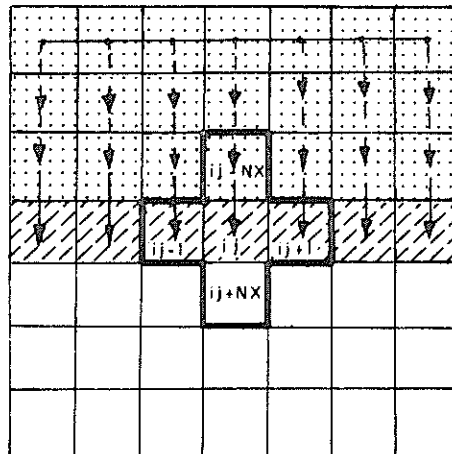


Figure 3:5. LSOR method.

The algorithm for LSOR is:

$$\begin{aligned}
 & C_W H_{ij-1}^{*k+1} - A H_{ij}^{*k+1} + C_E H_{ij+1}^{*k+1} \\
 &= \omega [r_{ij} - C_N H_{ij-NX}^{*k+1} - C_S H_{ij+NX}^{*k}] \\
 &+ (\omega - 1) [A H_{ij}^{*k} - C_W H_{ij-1}^{*k} - C_E H_{ij+1}^{*k}]
 \end{aligned} \tag{3:6}$$

Equation 3:6 consists of at most three unknowns ( $H_{ij}^{*k+1}$ ,  $H_{ij+1}^{*k+1}$ ,  $H_{ij-1}^{*k+1}$ ). A complete set of LSOR equations is then set up for a particular row and solved simultaneously by the use of Thomas Algorithm (explained below). This process is repeated row by row until the desired convergence is reached.

### Thomas Algorithm

Tridiagonal matrices result from one-dimensional problems (such as a row or a column). The tridiagonal is the main and the two adjacent diagonals. Equation 3:6 rewritten for each block of a row (or a column) becomes:

$$\begin{array}{rcl}
 b_1 u_1 + c_1 u_2 & & = d_1 \\
 a_2 u_1 + b_2 u_2 + c_2 u_3 & & = d_2 \\
 a_3 u_2 + b_3 u_3 + c_3 u_4 & & = d_3 \\
 \vdots & & \vdots \\
 \vdots & a_i u_{i-1} + b_i u_i + c_i u_{i+1} & = d_i \\
 \vdots & & \vdots \\
 & a_{n-1} u_{n-2} + b_{n-1} u_{n-1} + c_{n-1} u_n & = d_{n-1} \\
 & & \\
 & a_n u_{n-1} + b_n u_n & = d_n
 \end{array} \tag{3:7}$$

This system can be solved explicitly for the unknowns  $u_1, u_2, \dots, u_n$  thereby eliminating any matrix operations. The method is called the Thomas Algorithm (Rosenberg, 1969).

In general, the equations are:

$$a_i u_{i-1} + b_i u_i + c_i u_{i+1} = d_i \tag{3:8}$$

for  $1 \leq i \leq n$

with  $a_1 = c_n = 0$

$$\beta_i = b_i \frac{a_i c_{i-1}}{\beta_{i-1}} \tag{3:9}$$

with  $\beta_1 = b_1$

and

$$\gamma = \frac{d_i - a_i \gamma_{i-1}}{\beta_i} \quad (3:10)$$

with  $\gamma_i = \frac{d_i}{b_i}$ .

The values of the dependent variable are then computed from:

$$u_n = \gamma_n \quad (3:11)$$

$$u_i = \gamma_i - \frac{c_i u_{i+1}}{\beta_i} \quad (3:12)$$

This method has been shown to be stable for round-off error as long as  $|b_i| \geq |a_i + c_u|$  for each equation.

#### IV. DATA

For each grid, the following parameters have to be defined.

##### (1) Initial Water Table Elevation

The water table elevation, preferably to the nearest 0.1 foot, needs to be read from the water table contour map at each node of the grid system.

##### (2) Final Water Table Elevation

A map of final water table elevation is used to determine the average final water level of each grid. The figures obtained should have the same accuracy as those of the initial water table. These figures are not read into the computer but are used for comparison with the predicted result.

##### (3) Simulating Boundary Conditions

The boundaries of the study area are designated in this program by the following codes:

0 represents an aquifer node.

1 represents an impermeable boundary.

2 represents underflow; i.e., groundwater boundary inflow or outflow (gradient boundary condition). The rate of outflow as well as the rate of inflow ought to be computed (or estimated).



3 and 4 are constant head boundary conditions. Hydraulically connected surface water sources such as a river or lake may be simulated by programming a time-varying or constant water surface elevation in the appropriate grids. The value of 3 is used to represent this kind of boundary condition along model boundaries; otherwise, inside the model the value 4 is assigned.

(4) Bedrock Elevation

The bedrock elevation data are read into the computer initially and are kept constant for all time steps.

(5) Hydraulic Conductivity

The values of hydraulic conductivity can be obtained from pumping tests or specific capacity data. Since the model can handle anisotropy in x- and y-directions, as well as nonhomogeneity, hydraulic conductivities in x- and y-directions and as a function of location are read into the computer for each node and are held constant for all time steps.

(6) Storage Coefficient (or specific yield)

These values can also be obtained from pumping tests. In this model storage coefficient is a function of space only. Therefore, the values are read into the computer initially and held constant for all time steps.

(7) Ground Water Withdrawals

Groundwater pumping for irrigation, municipal, and industrial needs may have considerable effect upon water levels. Heavy pumping almost always causes marked declines in the water levels and results in a decrease somewhere in the natural discharge of ground water. Annual pumped volumes can be estimated from certain State reports and from declaration forms of owners. The average monthly distribution pumping figures are used to estimate the volume pumped in each time step.

The coefficient of pumping, the fraction of pumped water not returning to the aquifer when applied during usage, depends on the different usages of ground water.

(8) Precipitation

Average daily, monthly, and yearly precipitation data are available from published records of selected stations. For example, data for Socorro, New Mexico, shows that most of the precipitation occurs during the summer months in the form of local thunderstorms: light snowfalls in winter are

infrequent. The average annual precipitation (1920-1953) in this area is 8.6 inches and the period of greatest precipitation is from July to September. Soil moisture measurements by the Research and Development Division of the New Mexico Institute of Mining and Technology show that 85 percent of the precipitation from a relatively heavy storm is lost by evaporation or run-off. Losses of 10 percent occur due to the effects of climate, vegetation, and topography. The remaining 5 percent of precipitation is usually considered to infiltrate and reach the ground water. This is referred to as the coefficient of precipitation in this model.

The monthly distribution of annual precipitation and the coefficient of precipitation are read into the computer initially and held constant throughout the study period.

(9) Phreatophytes

Annual consumptive use by phreatophytes can be estimated from technical reports published by the State Engineer office. Many factors influence the amount of water consumed by plants. The important natural influences are climate, water supply, soil, and topography. A monthly step distribution of consumptive use is assumed. These figures are read into the computer initially and held constant for all time steps.

(10) Boundary Flow

Darcy's Law is used to calculate boundary inflow and outflow by estimating potential gradients, cross-sectional areas, and hydraulic conductivities. These values are read into the computer in acre-feet/year and held constant throughout all time steps.

(11) Water Leaking Into or Out of Aquifer

These values are read into the computer in acre-feet/year and held constant throughout all time steps. Leakage can be deduced from excessive differences between computed and historical water levels which cannot otherwise be explained.

(12) Water Seeping to Aquifer from Irrigation and Drainage Canals

These values are read into the computer in acre-feet/year and held constant during the irrigation season or varied as a function of the amount of water applied.







- (1) H = water table elevation (ft)
- (2) IBC = boundary condition tag
  - 0 - water table
  - 1 - impermeable boundary
  - 2 - inflow or outflow
  - 3 - constant head (along boundaries)
  - 4 - river or lake (within model boundaries)
- (3) PPT = precipitation (in/year)
- (4) RPUM = pumping rate (acre-feet/year)
- (5) RFREA = phreatophyte water use (acre-feet/year)
- (6) RLEAK = water leaking in or out (acre-feet/year)
- (7) RSEEP = water seeping to aquifer from irrigation canals (acre-feet/year)
- (8) QBC = inflow (+) or outflow (-) along model boundaries (acre-feet/year)
- (9) Z = bedrock elevation (ft)
- (10) G = ground surface elevation (ft)
- (11) FKX = hydraulic conductivity in X-direction (ft/day)
- (12) FKY = hydraulic conductivity in Y-direction (ft/day)
- (13) DX = finite increment in X-direction (ft)
- (14) DY = finite increment in Y-direction (ft)
- (15) PHI = effective porosity

Subroutine VALUES

This subroutine reads in:

- YPT = monthly precipitation distribution values
- YPM = monthly pumping distribution values
- YPR = monthly phreatophyte distribution values
- YHH = monthly variations of river stage
- CPT = the fraction of precipitation reaching ground water
- CPM = coefficient of pumping (effective pumping, consumed).

Subroutine COEF

This subroutine computes east and south coefficients of flow. (West and north coefficients are obtained by symmetry.)

Subroutine DISTRI

This subroutine combines distribution values according to time step size.

Subroutine QFIX

This subroutine computes the net withdrawal rates,  $Q$ , for each grid during each time increment as a result of pumping, precipitation, boundary inflow and outflow, phreatophyte use, water leakage, and water seepage.

Subroutine TRANSG

This subroutine computes the transmissivities for each grid during each time increment and sets them up in reduced matrix form for Gauss Elimination (see Section III).

Subroutine RESIDG

This subroutine computes the residual term of the  $k$ -th iterate, which lies between time level  $t$  and  $t+\Delta t$ .

Subroutine GAUS1

This subroutine calls for subroutine BSOLVE and checks for convergence.

Subroutine BSOLVE

This subroutine solves the matrix equation by Gauss Elimination.

Subroutine TRANSW

This subroutine computes the transmissivities for each grid during each time increment for the use of Line Successive over Relaxation Method.

Subroutine RESIDW

This subroutine computes the residuals which will be used in Line Successive over Relaxation Method.

Subroutine LSOR

This subroutine calls for subroutine THOMAS and solves all the equations using Line Successive over Relaxation Method.

Subroutine THOMAS

This subroutine solves a tridiagonal matrix by the "THOMAS algorithm."

Subroutine BALCOM

This subroutine computes the material balance for each time step.

Subroutine BDNFLO

This subroutine computes the boundary inflow and/or outflow.

### Subroutine MAP

This subroutine sets up different formats for printing corresponding arrays.

## VI. MODEL CHARACTERISTICS

### Size of Grid System

In this program, a blank common array A (see main program and subroutine OLAY1) is used to replace double-dimensional arrays. The biggest dimension allowed to be used for A in the IBM 360/44 computer is about 9000, and, therefore, the choice of optimum values for NX (No. of rows) and NY (No. of columns) is important.

Cards #10 to #31 in subroutine OLAY1 state the locations of all the arrays; consequently, the index ILAST will be the total number of storage locations required in the A array.

For Gaussian Elimination, some arrays are reallocated (cards #41 to #43). Card #43 states:

$$ILAST = IA16 + NX * NY * (2 * NY + 1) - 1 \quad (6:1)$$

which means that the last array needs  $NX * NY * (2 * NY + 1)$  words which is the largest array required. Apparently, much storage can be saved if NX is always greater than NY and should be considered when setting up the grid system.

Combining the information obtained from cards #10 to #31 and cards #41 to #43, the total number of words needed for the A array in case of Gauss Eliminations is:

$$\begin{aligned} & 20 * NX * NY + NX * NY * (2 * NY + 1) \\ & = NX * NY * (21 + 2 * NY) \end{aligned} \quad (6:2)$$

If this number is equated to the largest number allowed to be used for array A, then it will always give the maximum dimension of the system. That is,

$$NX * NY * (21 + 2 * NY) \leq 9000 \quad (6:3)$$



or

$$NX \leq \frac{9000}{(21 + 2 * NY) * NY} \quad (6:4)$$

For the LSOR method (some arrays are also reallocated [cards #51 to #67]), the array size can be calculated:

$$25 * NX * NY + 4 * NX + 2 * NY \leq 9000 \quad (6:5)$$

or

$$NX \leq \frac{9000 - 2 * NY}{4 + 25 * NY} \quad (6:6)$$

Table 6:1. Some maximum grid sizes for different NX and NY values.

NY	Gaussian Elimination		LSOR	
	NX	NX * NY	NX	NX * NY
5	54	270	69	345
10	21	210	36	360
15	11	165	33	345
20	7	140	17	340
25	5	125	14	350
30	3	90	11	330

Table 6:1 lists some maximum grid sizes for different values of NX and NY. It can be seen that LSOR is less restrictive on storage than Gauss Elimination and that it does not require that NX be greater than NY.

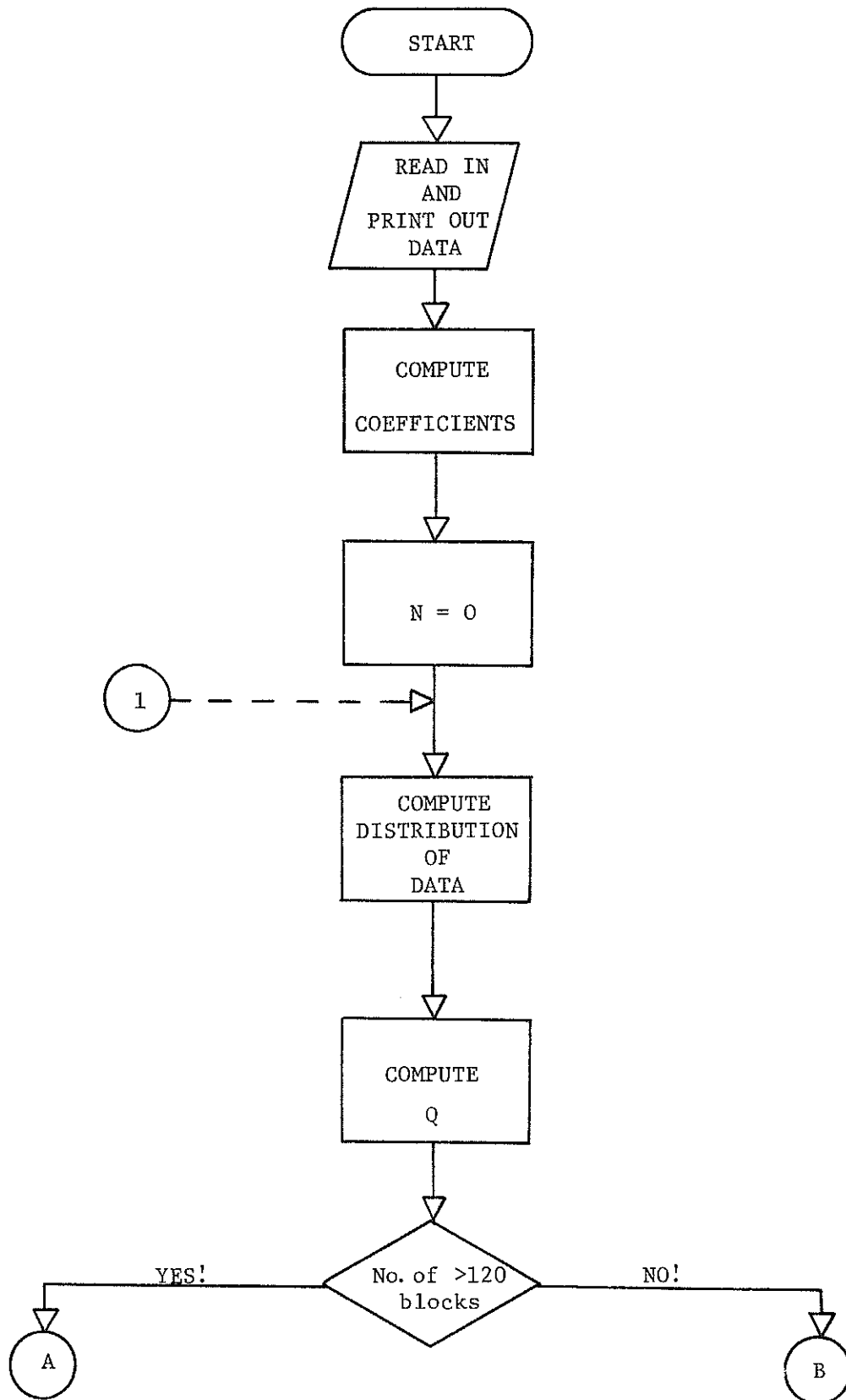
#### Time Step

For time steps smaller than one month, multiples of 5 days are required.

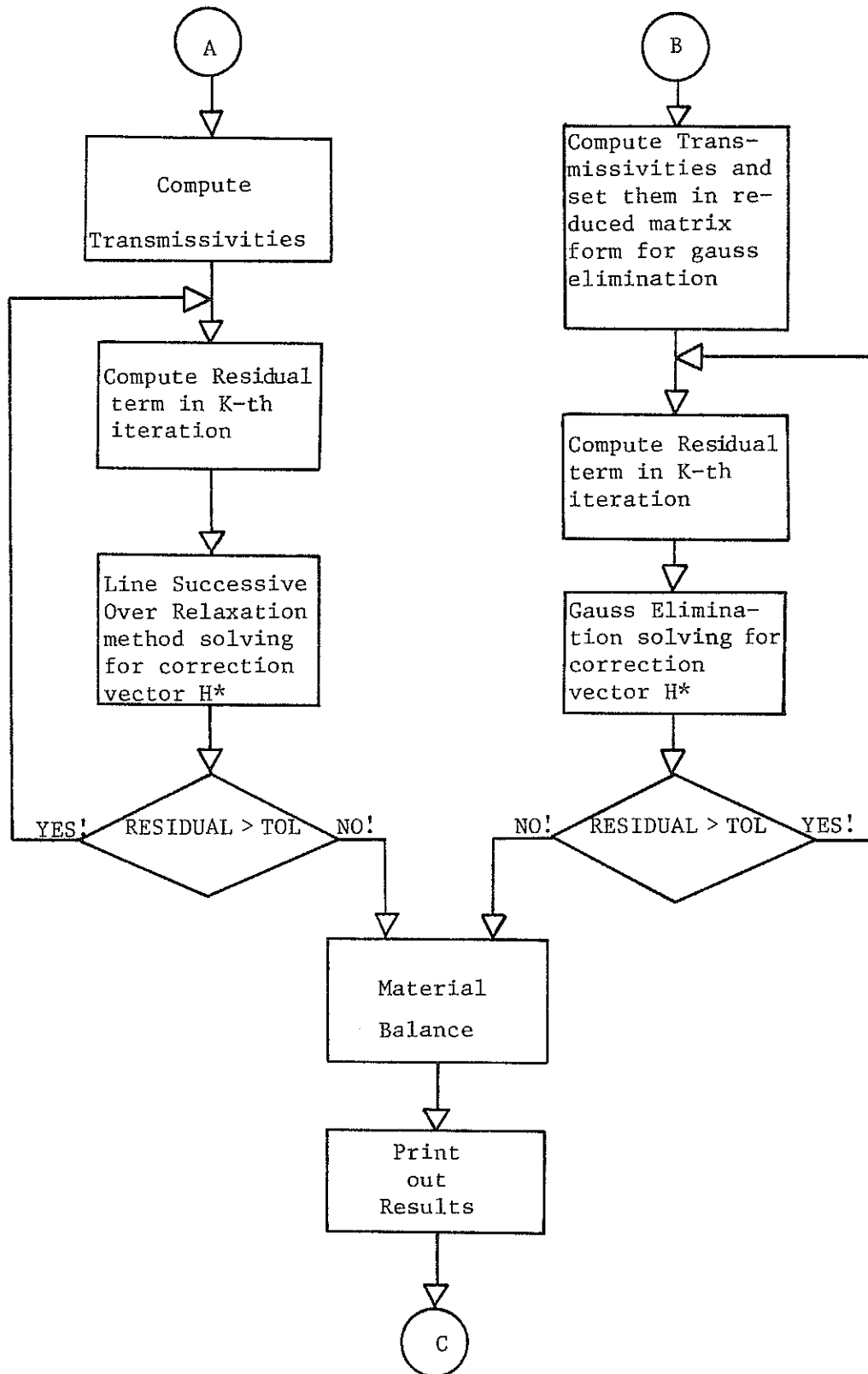
For time steps equal to or greater than 1 month, 1, 2, 3, 4, or 6 months can be utilized.

The largest time step is 1 year (or 365 days). The time step unit is in days.

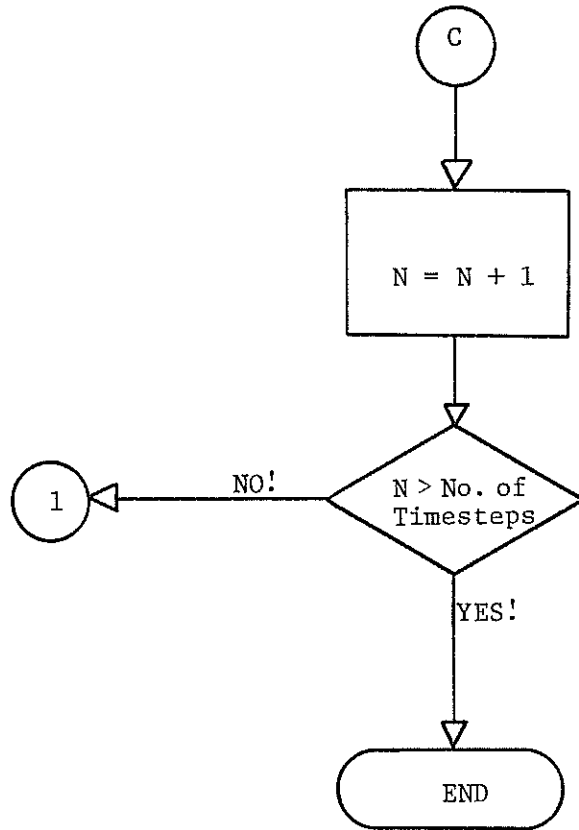
VII. FLOW CHART



(VII. cont.)



(VII. cont.)



## REFERENCES

- Lansford, R. R., S. Ben-David, T. Gebhard, Jr., W. Brutsaert, and B. J. Creel, *An Analytical Interdisciplinary Evaluation of the Utilization of the Water Resources of the Rio Grande in New Mexico*, New Mexico Water Resources Research Institute Report 020, 1973, 152 pp.
- \_\_\_\_\_, *An Analytical Interdisciplinary Evaluation of the Utilization of the Water Resources of the Rio Grande in New Mexico: Upper Rio Grande Region*, New Mexico Water Resources Research Institute Report 021, 1973.
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- Rosenberg, O., *Methods for the Numerical Solution of Partial Differential Equations*, American Elsevier Publishing Company, Inc., New York, 1969.
- Thurnau, D. H., "Algorithm 195, BANDSOLVE," *Communications of the ACM*, (1963), Vol. 6, No. 8, p. 441.

APPENDIX - COMPUTER LISTING

```

//SYS002 ACCESS S0S0J1,190='TCCRFS'
//MAIN EXEC FORTRAN(MAP)
C-----CONJUNCTIVE USE SURFACE WATER-GROUND WATER RESERVOIR SIMULATOR
COMMON A(9703)
COMMON/P)INT/NTAPE,NCOM,OMEGA,IGAU,IWAT,HCONV,IDEBUG,JAGAIN,
1 CUMTIM,CUMMB
COMMON/STOR/TSTOR,BIN,ROUT
NTAPE = 10
JAGAIN = 0
TSTOR = 0.0
CUMMB = 0.0
CUMTIM = 0.0
CALL OLAY1
END

```

```

/*
//OLAY1 EXEC FORTRAN(MAP)
SUBROUTINE OLAY1
COMMON A(9700)
COMMON/POINT/NTAPE,NCOM,OMEGA,IGAU,IWAT,HCONV,IDEBUG,JAGAIN,
1 CUMTIM,CUMMB
COMMON/DISTR/CPM,CCP,CPT,DYPM,DYPT,DYPR,DYHH,DYPS,
1 YPT(12),YPM(12),YPR(12),YHH(12),YPS(12)
CALL SINGLE (NTAPE,ABORT,NX,NY,DELTAT,OMEGA,IGAU,IWAT,
1 HCONV,NCOM,IDEBUG)
NXNY = NX*NY
NA = NX*NY
NB = 2*NY+1
ICOUNT = 0

```

-----ALLOCATION OF CORE FOR THE FOLLOWING SUBROUTINES IS AS FOLLOWS

ARRAY	SYMBOL	SIZE	DESCRIPTION
IA1	CE	{NXNY}	EFFECTIVE AREA (DX*DY*PHI) WATERTABLE ELEVATION BOUNDARY CONDITION TAG: 0 = WATERTABLE POINT 1 = IMPERMEABLE BOUNDARY 2 = INFLOW OR OUTFLOW 3 = CONSTANT HEAD (ALONG BOUNDARIES) 4 = RIVER OR LAKE (WITHIN MODEL BOUNDARIES) (=CONSTANT HEAD)
IA2	CS	{NXNY}	
IA3	FFAR	{NXNY}	
IA4	H	{NXNY}	
IA5	IBC	{NXNY}	
IA6	PPT	{NXNY}	RAINFALL (IN/YR)
IA7	RPJM	{NXNY}	PUMPING DISTRIBUTION (AC-FT/YR)
IA8	RFREA	{NXNY}	PHREATOPHYTE WATER USE (AC-FT/YR)
IA9	RLEAK	{NXNY}	WATER LEAKING IN OR OUT (AC-FT/YR)
IA10	RSEEP	{NXNY}	WATER SEEPING TO AQUIFER FROM IRRIG. CANALS
IA11	QBC	{NXNY}	INFLOW(+) OR OUTFLOW ALONG MODEL BOUNDARIES (AC-FT/YR)
IA12	Z	{NXNY}	BEDROCK ELEVATION (FT)
IA13	G	{NXNY}	GROUND SURFACE ELEVATION (FT)
IA14	HP	{NXNY}	WATER TABLE FLEV. OF PREVIOUS TIME STEP (FT)
IA15	Q	{NXNY}	NET RATE OF WITHDRAWAL (AC-FT/DAY)
IA16	FKX	{NXNY}	HYDRAULIC CONDUCTIVITY IN X-DIR. (FT/DAY)
IA17	FKY	{NXNY}	HYDRAULIC CONDUCTIVITY IN Y-DIR. (FT/DAY)
IA18	DX	{NXNY}	FINITE INCREMENTS IN X-DIRECTION
IA19	DY	{NXNY}	FINITE INCREMENTS IN Y-DIRECTION
IA20	PHI	{NXNY}	EFFECTIVE POROSITY







```

C      CALL RESIDW (NX, NY, A(IA15), A(IA4), A(IA5), A(IA3), JAGAIN, IDEBUG,
C      1          DELTAT, A(IA16), A(IA17), A(IA18), A(IA19), A(IA20),
C      2          A(IA30), A(IA14), CUMTIM)
C
C      CALL LSOR (A(IA16), A(IA17), A(IA18), A(IA19), A(IA20), A(IA22),
C      1          A(IA30), A(IA25), A(IA26), A(IA27), A(IA28), NX, NY,
C      2          OMEGA, IDEBUG, CUMTIM, A(IA4), A(IA5), A(IA3), A(IA14),
C      3          HCONV, ITER, DELTAT, JAGAIN, ICOUNT)
C
C      IF (ITER.EQ.1) GO TO 401
300  CONTINUE
C
C
C      CALL BALCOM (NX, NY, A(IA4), A(IA14), A(IA3), A(IA5), A(IA12), A(IA13),
C      1          A(IA1), A(IA2), DELTAT, CUMTIM, CUMMP, SQVOL, ICOUNT)
C      ICOUNT = ICOUNT + 1
C      IF (ICOUNT.LT.NCOM) GO TO 500
C      RETURN
C      END

```

```

/*
//SINGLE EXEC FORTRAN(MAP)
SUBROUTINE SINGLE (NTAPE, APORT, NX, NY, DELTAT, OMEGA, IGAU, IWAT,
1          HCONV, NCOM, IDEBUG)

```

```

C--VARIABLE DESIGNATION AS USED
C      IN PROGRAM          ON CARDS          IN DATA STAT.
C-----
C      NCOM                CMNT                KCMN
C      OMEGA                NCOM                KNCO
C      IGAU                 OMEG                KOME
C      IWAT                 IGAU                KIGA
C      HCONV                IWAT                KIWA
C      IDEBUG               HCONV               KHCO
C      DELTAT               DEBU                KDEB
C      NX                   DELT                KDEL
C      NY                   NX                  KNX
C                          NY                  KNY
C                          ENDS                KEND

```

```

1  DATA KCMN/'CMNT'//,KNCO/'NCOM'//,KOME/'OMEG'//,KIGA/'IGAU'//,
2  KIWA/'IWAT'//,KHCO/'HCON'//,KDEB/'DEBU'//,KDEL/'DELT'//,
   KNX/'NX' //,KNY/'NY' //,KEND/'ENDS'//

```

```

C      10  READ (5,1) NAME,X1
C      IF (NAME.EQ.KCMN) GO TO 10
C      IF (NAME.EQ.KEND) GO TO 999
C      IF (NAME.EQ.KNCO) GO TO 100
C      IF (NAME.EQ.KOME) GO TO 200
C      IF (NAME.EQ.KIGA) GO TO 300
C      IF (NAME.EQ.KIWA) GO TO 400
C      IF (NAME.EQ.KHCO) GO TO 500
C      IF (NAME.EQ.KDEB) GO TO 600
C      IF (NAME.EQ.KDEL) GO TO 700
C      IF (NAME.EQ.KNX) GO TO 800
C      IF (NAME.EQ.KNY) GO TO 900

```

```

C 100 NCOM = X1
      GO TO 10
C 200 OMEGA = X1
      GO TO 10
C 300 IGAU = X1
      GO TO 10
C 400 IWAT = X1
      GO TO 10
C 500 HCONV = X1
      GO TO 10
C 600 IDFBUG = X1
      GO TO 10
C 700 DELTAT = X1
      GO TO 10
C 800 NX = X1
      GO TO 10
C 900 NY = X1
      GO TO 10
C 999 CONTINUE
      FORMAT (A4,6X,F10.0)
      RETURN
      END

```

```

/*
//ARRAYS EXFC FORTRAN(MAP)
SUBROUTINE ARRAYS (NX, NY, H, IBC, PPT, RPUM, RFRFA, RLEAK, RSEEP, QBC, Z,
1 G, HP, FKX, FKY, DX, DY, PHI, CUMTIM)
1 DIMENSION H(1), IBC(1), PPT(1), RPUM(1), RFRFA(1), RLEAK(1), RSEEP(1),
1 QBC(1), Z(1), G(1), HP(1), FKX(1), FKY(1), DX(1), DY(1), PHI(1)
C--VARIABLE DESIGNATION AS USED

```

	IN PROGRAM	ON CARDS	IN DATA STAT.
	H	CMNT	KCMN
	IBC	H	KH
	PPT	IBC	KIBC
	RPUM	PPT	KPPT
	RFRFA	RPUM	KPUM
	RLEAK	RFRFA	KFRE
	RSEEP	LEAK	KLEA
	QBC	RSEEP	KSFE
	Z	QBC	KQBC
	G	Z	KZ
	FKX	G	KG
	FKY	FKX	KFKX
	DX	FKY	KFKY
	DY	DX	KDX
	PHI	DY	KDY
		PHI	KPHI
		ENDA	KEND

```

C
C DATA KCMN/'CMNT'//,KH // 'H' //,KIBC/'IBC' //,KPPT/'PPT' //,
1 KPUM/'PUM' //,KFRE/'FRE' //,KLEA/'LEAK' //,KSEE/'SEEP' //,
2 KQBC/'QBC' //,KZ // 'Z' //,KG // 'G' //,KFKX/'FKX' //,
3 KFKY/'FKY' //,KDX // 'DX' //,KDY // 'DY' //,KPHI/'PHI' //,
4 KEND/'ENDA'//
C
C NXNY = NX*NY
C 1 READ (5,10) NAME,X1,X2
C

```

```

IF (NAME.EQ.KCMJ) GO TO 1
IF (NAME.EQ.KEND) GO TO 200
IF (NAME.EQ.KH ) GO TO 100
IF (NAME.EQ.KIBC) GO TO 200
IF (NAME.EQ.KPHI) GO TO 300
IF (NAME.EQ.KPUM) GO TO 400
IF (NAME.EQ.KFRE) GO TO 500
IF (NAME.EQ.KLEA) GO TO 600
IF (NAME.EQ.KSEE) GO TO 700
IF (NAME.EQ.KQBC) GO TO 800
IF (NAME.EQ.KZ ) GO TO 900
IF (NAME.EQ.KG ) GO TO 1000
IF (NAME.EQ.KFKX) GO TO 1100
IF (NAME.EQ.KFKY) GO TO 1200
IF (NAME.EQ.KDX ) GO TO 1300
IF (NAME.EQ.KDY ) GO TO 1400
IF (NAME.EQ.KPPT) GO TO 1500

C
100 IF (X1.NE.0.) GO TO 101
DO 11 I=1,NXNY
H(I) = X2
11 HP(I) = H(I)
GO TO 1
101 DO 102 J=1,NY
K1 = NX*(J-1)+1
K2 = K1+NX-1
102 READ (5,2) (H(K),K=K1,K2)
DO 3 I=1,NXNY
HP(I) = H(I)
GO TO 1

C
200 IF (X1.NE.0.) GO TO 201
DO 22 I=1,NXNY
IBC(I) = X2
GO TO 1
201 DO 202 J=1,NY
K1 = NX*(J-1)+1
K2 = K1+NX-1
202 READ(5,*) (IBC(K),K=K1,K2)
GO TO 1

C
300 IF (X1.NE.0.) GO TO 301
DO 33 I=1,NXNY
PHI(I) = X2
GO TO 1
301 DO 302 J=1,NY
K1 = NX*(J-1)+1
K2 = K1+NX-1
302 READ (5,2) (PHI(K),K=K1,K2)
GO TO 1
400 IF (X1.NE.0.) GO TO 401
DO 44 I=1,NXNY
RPUM(I) = X2
GO TO 1
401 DO 402 J=1,NY
K1 = NX*(J-1)+1
K2 = K1+NX-1
402 READ (5,2) (RPUM(K),K=K1,K2)
GO TO 1

C
500 IF (X1.NE.0.) GO TO 501
DO 55 I=1,NXNY
RFREA(I) = X2
GO TO 1
501 DO 502 J=1,NY
K1 = NX*(J-1)+1
K2 = K1+NX-1
502 READ (5,2) (RFREA(K),K=K1,K2)
GO TO 1

C
600 IF (X1.NE.0.) GO TO 601
DO 66 I=1,NXNY
RLEAK(I) = X2
GO TO 1
601 DO 602 J=1,NY
K1 = NX*(J-1)+1
K2 = K1+NX-1
602 READ (5,2) (RLEAK(K),K=K1,K2)
GO TO 1

C

```

```

700 IF (X1.NE.0.) GO TO 701
    DO 77 I=1,NXNY
77   RSEEP(I) = X2
    GO TO 1
701 DO 702 J=1,NY
      K1 = NX*(J-1)+1
      K2 = K1+NX-1
702 READ (5,2) (RSEEP(K),K=K1,K2)
    GO TO 1
C
800 IF (X1.NE.0.) GO TO 801
    DO 88 I=1,NXNY
88   QBC(I) = X2
    GO TO 1
801 DO 802 J=1,NY
      K1 = NX*(J-1)+1
      K2 = K1+NX-1
802 READ (5,2) (QBC(K),K=K1,K2)
    GO TO 1
C
900 IF (X1.NE.0.) GO TO 901
    DO 99 I=1,NXNY
99   Z(I) = X2
    GO TO 1
901 DO 902 J=1,NY
      K1 = NX*(J-1)+1
      K2 = K1+NX-1
902 READ (5,2) (Z(K),K=K1,K2)
    GO TO 1
C
1000 IF (X1.NE.0.) GO TO 1001
    DO 111 I=1,NXNY
111  G(I) = X2
    GO TO 1
1001 DO 1002 J=1,NY
      K1 = NX*(J-1)+1
      K2 = K1+NX-1
1002 READ (5,2) (G(K),K=K1,K2)
    GO TO 1
C
1100 IF (X1.NE.0.) GO TO 1101
    DO 112 I=1,NXNY
112  FKX(I) = X2
    GO TO 1
1101 DO 1102 J=1,NY
      K1 = NX*(J-1)+1
      K2 = K1+NX-1
1102 READ (5,2) (FKX(K),K=K1,K2)
    GO TO 1
C
1200 IF (X1.NE.0.) GO TO 1201
    DO 123 I=1,NXNY
123  FKY(I) = X2
    GO TO 1
1201 DO 1202 J=1,NY
      K1 = NX*(J-1)+1
      K2 = K1+NX-1
1202 READ (5,2) (FKY(K),K=K1,K2)
    GO TO 1
C
1300 IF (X1.NE.0.) GO TO 1301
    DO 134 I=1,NXNY
134  DX(I) = X2
    GO TO 1
1301 DO 1302 J=1,NY
      K1 = NX*(J-1)+1
      K2 = K1+NX-1
1302 READ (5,2) (DX(K),K=K1,K2)
    GO TO 1
C
1400 IF (X1.NE.0.) GO TO 1401
    DO 145 I=1,NXNY
145  DY(I) = X2
    GO TO 1
1401 DO 1402 J=1,NY
      K1 = NX*(J-1)+1
      K2 = K1+NX-1
1402 READ (5,2) (DY(K),K=K1,K2)
    GO TO 1
C

```

```

1500 IF (X1.NE.0.) GO TO 1501
DO 156 I=1,NX
DO 156 J=1,NY
IJ = NX*(J-1)+I
PPT(IJ) = X2
PPT(IJ) = PPT(IJ)*DX(IJ)*DY(IJ)/(43560.*12.)
156 CONTINUE
GO TO 1
1501 DO 1502 J=1,NY
K1 = NX*(J-1)+1
K2 = K1+NX-1
1502 READ (5,2) (PPT(K),K=K1,K2)
DO 256 IJ=1,NX*NY
PPT(IJ) = PPT(IJ)*DX(IJ)*DY(IJ)/(43560.*12.)
256 GO TO 1
C
9999 CONTINUE
C-----PRINT OUT DATA
CALL MAP (H,4HH ,2.,NX,NY,CUMTIM)
CALL MAP (HP,4HHP ,2.,NX,NY,CUMTIM)
CALL MAP (PPT,4HPPT ,2.,NX,NY,CUMTIM)
CALL MAP (RPIJM,4HPJM ,2.,NX,NY,CUMTIM)
CALL MAP (RFREA,4HFA ,2.,NX,NY,CUMTIM)
CALL MAP (RLEAK,4HLEAK ,2.,NX,NY,CUMTIM)
CALL MAP (RSEEP,4HSEEP ,2.,NX,NY,CUMTIM)
CALL MAP (QRC,4HQRC ,2.,NX,NY,CUMTIM)
CALL MAP (FKX,4HFKX ,2.,NX,NY,CUMTIM)
CALL MAP (FKY,4HFKY ,2.,NX,NY,CUMTIM)
CALL MAP (DX,4HDX ,2.,NX,NY,CUMTIM)
CALL MAP (DY,4HDY ,2.,NX,NY,CUMTIM)
CALL MAP (PHI,4HPhi ,2.,NX,NY,CUMTIM)
CALL MAP (G,4HG ,2.,NX,NY,CUMTIM)
CALL MAP (Z,4HZ ,2.,NX,NY,CUMTIM)
PRINT 3000, CUMTIM
3000 FORMAT (1H)/40X,19HBOUNDARY CONDITIONS,2X,3HMAP,20X,7HTIME = ,
1 F10.4,2X,4HDAYS/
DO 5000 J=1,NY
K1 = NX*(J-1) + 1
K2 = K1 + NX - 1
5000 WRITE (6,2000) (IBC(K),K=K1,K2)
2000 FORMAT (30I4)
2 FORMAT (7F10.0)
4 FORMAT (7I10)
10 FORMAT (A4,6X,2F10.0)
RETURN
END

/*
//VALUES EXEC FORTRAN(MAP)
SUBROUTINE VALUES (YPT,YPM,YPR,YHH,YPS,CPT,CPM)
DIMENSION YPT(12),YPM(12),YPR(12),YHH(12),YPS(12)
C---THIS SUBROUTINE READS IN
C YPT = PRECIPITATION DISTRIBUTION VALUES
C YPM = PUMPING DISTRIBUTION VALUES
C YPR = PHREATOPHYTE DISTRIBUTION VALUES
C YHH = VARIATIONS OF RIVER STAGES
C YPS = SEEPAGE DISTRIBUTION VALUES
C CPT = THE FRACTION OF PRECIPITATION REACHING GROUND WATER
C CPM = COEFFICIENT OF PUMPING (EFFECTIVE PUMPING, CONSUMED)
C
DATA KC4N/'CNT'//,KYPT/'YPT' //,KYPM/'YPM' //,KCPT/'CPT' //,
1 KCPM/'CPM' //,KENDV/'ENDV'//,KYPR/'YPR' //,KYHH/'YHH' //,
2 KYPS/'YPS' //
C
10 READ (5,1) NAME,X1
C
IF (NAME.EQ.KC4N) GO TO 10
IF (NAME.EQ.KENDV) GO TO 200
IF (NAME.EQ.KYPT) GO TO 100
IF (NAME.EQ.KYPM) GO TO 200
IF (NAME.EQ.KYPR) GO TO 300
IF (NAME.EQ.KYHH) GO TO 400
IF (NAME.EQ.KYPS) GO TO 500
IF (NAME.EQ.KCPT) GO TO 600
IF (NAME.EQ.KCPM) GO TO 700
C

```

```

100 READ (5,2) (YPT(I),I=1,12)
   GO TO 10
C
200 READ (5,2) (YPM(I),I=1,12)
   GO TO 10
C
300 READ (5,2) (YPR(I),I=1,12)
   GO TO 10
C
400 READ (5,2) (YHH(I),I=1,12)
   GO TO 10
C
500 READ (5,2) (YPS(I),I=1,12)
   GO TO 10
C
600 CPT = X1
   GO TO 10
C
700 CPM = X1
   GO TO 10
C
999 CONTINUE
   1 FORMAT (14,6X,F10.0)
   2 FORMAT (7F10.0)
   RETURN
   END

```

```

/*
//COEF EXEC FORTRAN(MAP)
SUBROUTINE COEF (NX,NY,PHI,DX,DY,FFAR,FKX,FKY,CE,CS,IBC,CUMTIM,
1          IDEFUG)
1 DIMENSION PHI(1),DX(1),DY(1),FFAR(1),FKX(1),FKY(1),CE(1),CS(1),
1          IBC(1)
1 REAL*8 DEFAR,DCE,DCS,DDX,DDY,DFKX,DFKY,DDX1,DDXNX,DFKX1,DFKXNX,
1          DDY1,DDYNX,DFKY1,DFKYNX,DPHI
C----- THIS SUBROUTINE COMPUTES THE EAST AND SOUTH COEFFICIENTS OF THE FLOW
C----- EQUATION

```

```

C
DO 100 I = 1,NX
  DO 100 J = 1,NY
    IJ = NX*(J-1)+I
    DPHI = PHI(IJ)
    DDY = DY(IJ)
    DDX = DX(IJ)
    IF (I.EQ.1.OR.I.EQ.NX) DDX = DDX/2.
    CE(IJ) = 0.0
    CS(IJ) = 0.0
    DEFAR = DDX*DDY*DPHI
    FFAR(IJ) = DEFAR
    DDX = DX(IJ)
    DDX1 = DX(IJ+1)
    DDXNX = DX(IJ+NX)
    DFKX = FKX(IJ)
    DFKY = FKY(IJ)
    DFKX1 = FKX(IJ+1)
    DFKXNX = FKX(IJ+NX)
    DFKY1 = FKY(IJ+1)
    DFKYNX = FKY(IJ+NX)
    DDY1 = DY(IJ+1)
    DDYNX = DY(IJ+NX)
    IF (FKX(IJ).EQ.0.0.OR.FKY(IJ).EQ.0.0) FFAR(IJ)=0.0
    IF (I.EQ.NX) GO TO 50
    IF (FKX(IJ).EQ.0.0.OR.FKX(IJ+1).EQ.0.0) GO TO 50
    DCE = 2.*(DFKX*DFKX1*DDY*DDY1/
1          (DDX*DDY1*DFKX1+DDX1*DFKX*DDY)
    CE(IJ) = DCE
    50 IF (J.EQ.NY) GO TO 100
    IF (FKY(IJ).EQ.0.0.OR.FKY(IJ+NX).EQ.0.0) GO TO 100
    DCS = 2.*(DFKY*DFKYNX*DDX*DDXNX/
1          (DDY*DFKYNX*DDXNX+DDYNX*DFKY*DDX)
    CS(IJ) = DCS
  100 CONTINUE
C
IF (IDEFUG.EQ.1)
1CALL MAP (FFAR,4,HEFAR,2.,NX,NY,CUMTIM)
IF (IDEFUG.EQ.1)
1CALL MAP (CE,4,HCE ,2.,NX,NY,CUMTIM)
IF (IDEFUG.EQ.1)
1CALL MAP (CS,4,HCS ,2.,NX,NY,CUMTIM)
RETURN
END

```



```

/*
//DISTR EXEC FORTRAN(MAP)
SUBROUTINE DISTR (NX,NY,FPT,PPUM,RFREA,H,IBC,YPM,YPT,
1 YPR,YHH,YPS,DELTAT,ICOUNT,DYPM,DYPT,DYPR,DYHH,
2 DYPS)
DIMENSION PPT(1),RPUY(1),RFREA(1),H(1),IBC(1),YHH(12),
1 YPM(12),YPT(12),YPR(12),YPS(12)
C-----THIS SUBROUTINE COMPUTES THE DISTRIBUTION OF DATA.
C
JCOUNT = ICOUNT + 1
KD = JCOUNT
I2 = 365.2/DELTAT
1 IF (KD.LE.I2) GO TO 3
KD = KD - I2
GO TO 1
3 CONTINUE
C
IF (I2.LT.12) GO TO 6
I2 = I2/12
I = (KD-1)/I3 + 1
DYPM = YPM(I)/I3
DYPT = YPT(I)/I3
DYPR = YPR(I)/I3
DYHH = YHH(I)/I3
DYPS = YPS(I)/I3
GO TO 7
C
6 I4 = 12/I2
DYPM = 0.0
DYPT = 0.0
DYPR = 0.0
DYHH = 0.0
DYPS = 0.0
N1 = (KD-1)*I4+1
N2 = N1+I4-1
DO 5 I=N1,N2
DYPM = DYPM+YPM(I)
DYPT = DYPT+YPT(I)
DYPR = DYPR+YPR(I)
DYHH = DYHH+YHH(I)
DYPS = DYPS+YPS(I)
5 CONTINUE
7 CONTINUE
C
DO 4 I=1,NX
DO 4 J=1,NY
IJ = NX*(J-1)+I
IF (IBC(IJ).NE.4) GO TO 4
H(IJ) = H(IJ) + DYHH
4 CONTINUE
RETURN
END

```

```

/*
//QFIX EXEC FORTRAN(MAP)
SUBROUTINE QFIX (NX,NY,H,PPT,PPUM,RFREA,RLEAK,RSFEP,QBC,Z,Q,
1 DELTAT,IWAT,SQVOL,DYPM,DYPT,DYPR,DYPS,CPT,CPM,
2 CUMTIM)
DIMENSION H(1),PPT(1),RPUY(1),RFREA(1),RLEAK(1),RSFEP(1),QBC(1),
1 Z(1),Q(1)
COMMON/STOR/TSOR,BIN,BOUT
C-----THIS SUBROUTINE COMPUTES THE NET RATES OF APPLICATION, Q(NT), FOR EACH BLOCK
C THE COMPUTATION OF Q(NT) SHOULD COME OUT IN ACRE-FT/DAY
C
BIN = 0.0
BOUT = 0.0
SQVOL = 0.0
NT = 0
C
DO 4 I=1,NX
DO 4 J=1,NY
IJ = NX*(J-1)+I
NT = NT+1
IF (IWAT.EQ.1) GO TO 3
Q(NT) = 0.0
Q(NT) = -RPUY(IJ)*DYPM*CPM/DELTAT
3
4

```

```

1          +PPT(IJ)*DYPT*CPT/DEL TAT
2          +QRC(IJ)/365.
3          -RFREA(IJ)*DYPR/DEL TAT
4          -RLEAK(IJ)/365.
5          +RSEFP(IJ)*DYPS/DEL TAT
          SQVOL = SQVOL + Q(NT) - QRC(IJ)/365.
          IF (QRC(IJ).GE.0.0) BIN = BIN + QRC(IJ)/365.
          IF (QRC(IJ).LT.0.0) BOUT = BOUT + QRC(IJ)/365.
          GO TO 4
C
3          Q(IJ) = 0.0
          Q(IJ) = -RPJM(IJ)*DYPM*CPM/DEL TAT
1          +PPT(IJ)*DYPT*CPT/DEL TAT
2          +QRC(IJ)/365.
3          -RFREA(IJ)*DYPR/DEL TAT
4          -RLEAK(IJ)/365.
5          +RSEFP(IJ)*DYPS/DEL TAT
          SQVOL = SQVOL + Q(IJ) - QRC(IJ)/365.
          IF (QRC(IJ).GE.0.0) BIN = BIN + QRC(IJ)/365.
          IF (QRC(IJ).LT.0.0) BOUT = BOUT + QRC(IJ)/365.
4          CONTINUE
          RETURN
          END

```

```

/*
//RESIDG EXEC FORTRAN(MAP)
SUBROUTINE RESIDG (NX,NY,CMATRX,CR,H,IBC,FFAR,HP,NA,NB,
1          JAGAIN,IDFRUG,DEL TAT)
DIMENSION CMATRX(NA,NB),CR(1),H(1),IBC(1),FFAR(1),HP(1)
REAL*8 DC1,DCIC,DCIB,DCID,DCIM,DH1,DHIC,DHIB,DHID,DHIM,DCR,
1          DE,DEFAR,DCT,DHPIM
C----- THIS SUBROUTINE COMPUTES THE RESIDUAL TERM OF THE K-TH ITERATE, DEFINED AS
C          A)*H(I,J)-AN*H(I,J-1)-AS*H(I,J+1)-AW*H(I-1,J)-AE*H(I+1,J)
C          +RHS(IJ) = R(IJ)
C          WHERE RHS IS THE RIGHT HAND SIDE OF THE FINITE DIFFERENCES EQUATION
C          RHS(IJ) = -(PHI(IJ)*DX(IJ)*DY(IJ)/DEL TAT)*HP(IJ) + Q(IJ)*43560.
C          HP IS THE WATERTABLE ELEVATION AT TIME LEVEL T
C          THE K-TH ITERATE LIES BETWEEN TIME LEVEL T AND TIME LEVEL T+DEL TAT
C
          NXNY = NX*NY
          IB = NY
          IM = IB+1
          IC = IM+1
          ID = NY
          IF (JAGAIN.EQ.1) GO TO 1
          DO 2 J=1,NB
            DO 3 I=1,NA
2          CMATRX(I,J) = 0.0
          REWIND 1
          READ(1) (CMATRX(I,1),CMATRX(I,IB),CMATRX(I,IC),CMATRX(I,ID),
1          CMATRX(I,IM),CR(I),I=1,NXNY)
          CONTINUE
          NT = 0
C
          DO 10 I=1,NX
            DO 10 J=1,NY
              NT = NT + 1
              IJ = NX*(J-1)+I
              IF (IBC(IJ).GE.3) GO TO 5
              DC1 = CMATRX(NT,1)
              DCIC = CMATRX(NT,IC)
              DCIB = CMATRX(NT,IB)
              DCID = CMATRX(NT,ID)
              DCIM = CMATRX(NT,IM)
              DH1 = 0.
              DHIC = 0.
              DHIB = 0.
              DHID = 0.
              DHIM = H(IJ)
              IF (I.EQ.1) GO TO 11
              DH1 = H(IJ-1)
              DH1 = DHIM - DH1
11             IF (J.EQ.NY) GO TO 12
              DHIC = H(IJ+NX)
              DHIC = DHIM - DHIC
12             IF (J.EQ.1) GO TO 13
              DHIB = H(IJ-NX)
              DHIB = DHIM - DHIB
13             IF (I.EQ.NX) GO TO 14

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```

14      DHID = H(IJ+1)
        DHID = DHIM - DHID
        DEFAR = EFAR(IJ)
        DDT = DELTAT
        DHPIM = HP(IJ)
        DCR = CR(NT)
        DCR = DCR*43560.
        DE = (DEFAR/DDT)*(DHIM-DHPIM)
        DCR = DCR + DE
        DCR = -(DC1*DH1 + DCIC*DHIC + DCIB*DHIB + DCID*DHID) +
1          DCR
        CR(NT) = -DCR
        IF (EFAR(IJ).EQ.0.0) CR(NT) = 0.0
        IF (EFAR(IJ).EQ.0.0) CMATRX(NT,IM)=1.0
        GO TO 10
5      CMATRX(NT,1) = 0.
        CMATRX(NT,IB) = 0.
        CMATRX(NT,IC) = 0.
        CMATRX(NT,ID) = 0.
        CMATRX(NT,IM) = 1.0
        CR(NT) = 0.
10     CONTINUE
C     IF (IDDEBUG.EQ.1)
1     IWRITE (6,50)
        IF (IDDEBUG.EQ.1)
1     PRINT 60, (CMATRX(NT,1),CMATRX(NT,IB),CMATRX(NT,IM),
2           CMATRX(NT,IC),CMATRX(NT,ID),CR(NT),NT=1,NXNY)
50    FORMAT (1H1,9X,12HCMATRX(NT,1),5X,13HCMATRX(NT,IB),3X,
1     13HCMATRX(NT,IM),5X,13HCMATRX(NT,IC),6X,13HCMATRX(NT,ID),
2     7X,6HCR(NT),5X,3H(RESIDG)//)
60    FORMAT (6F19.7)
        RETURN
        END

```

```

/*
//TRANSG EXEC FORTRAN(MAP)
SUBROUTINE TRANSG (CR,CMATRX,EFAR,HP,Z,CF,CS,IBC,NX,NY,DELTAT,
1     NA,IB,DEBUG)
1     DIMENSION CR(1),CMATRX(NA,NB),EFAR(1),HP(1),Z(1),CE(1),CS(1),
1     IBC(1)
1     REAL*8 DAW,DAE,DAN,DAS,DCMAT,DF,DEFFAR,DDT
C-----THIS SUBROUTINE COMPUTES THE TRANSMISSIVITIES AND SETS THEM UP IN
C-----REDUCED MATRIX FORM FOR GAUSS ELIMINATION (BSOLVE)
C
        NXNY = NX*NY
        NA = NXNY
        NB = 2*NY + 1
        DO 1 J=1,NB
            DO 1 I=1,NA
1         CMATRX(I,J) = 0.0
            IB = NY
            IM = IB + 1
            IC = IM + 1
            ID = NB
            NT = 0
C
        DO 100 I=1,NX
            DO 100 J=1,NY
                IJ = NX*(J-1)+1
                NT = NT + 1
                AW = 0.0
                AV = 0.0
                AE = 0.0
                AS = 0.0
                DAW = 0.0
                DAE = 0.0
                DAN = 0.0
                DAS = 0.0
                HIJ = HP(IJ)
                IF (I.EQ.1) GO TO 3
                CCW = CE(IJ-1)
                AW=CCW*(AMAX1(HP(IJ),HP(IJ-1))-AMAX1(Z(IJ),Z(IJ-1)))
                CMATRX(NT,1) = -AW
                DAW = AW

```

```

3     IF (I.EQ.NX) GO TO 4
        CMATRX(NT,IO) = -AE
        CCE = CE(IJ)
        AE=CCE*(AMAX1(HP(IJ),HP(IJ+1))-AMAX1(Z(IJ),Z(IJ+1)))
        DAE = AE
4     IF (J.EQ.1) GO TO 5
        CCN = CS(IJ-NX)
        AN=CCN*(AMAX1(HIJ,HP(IJ-NX))-AMAX1(Z(IJ),Z(IJ-NX)))
        CMATRX(NT,IB) = -AN
        DAN = AN
5     IF (J.EQ.NY) GO TO 6
        CCS = CS(IJ)
        AS=CCS*(AMAX1(HIJ,HP(IJ+NX))-AMAX1(Z(IJ),Z(IJ+NX)))
        CMATRX(NT,IC) = -AS
        DAS = AS
6     DDT = DELTAT
        DEFAR = EFAR(IJ)
        DE = DFFAR/DDT
        DCMAT = DAW + DAN + DAS + DAE + DE
        CMATRX(NT,IM) = DCMAT
C---FOLLOWING 'IF' STATEMENT FOR IMPERMEABLE GRID BLOCKS ONLY.
C---UP TO THIS POINT CR HAS THE Q VALUES STORED; IN QFIX, Q SHOULD BE STORED
C---AS CR.
        CR(NT) = -CR(NT)
100  CONTINUE
C
    IF (IDEBUG.EQ.1)
1CALL MAP (EFAR,4,HEFAR,2.,NX,NY,CUMTIM)
    IF (IDFBUG.EQ.1)
1WRITE (5,50)
    IF (IDEBUG.EQ.1)
1PRINT 60,(CMATRX(NT,1),CMATRX(NT,IB),CMATRX(NT,IM),
2      CMATRX(NT,IC),CMATRX(NT,IO),CR(NT),NT=1,NXNY)
50  FORMAT (1H1,8X,12HC MATRX(NT,1),5X,13HC MATRX(NT,IB),3X,
1      13HC MATRX(NT,IM),5X,13HC MATRX(NT,IC),6X,13HC MATRX(NT,IO),
2      7X,6HCR(NT),5X,8H(TRANSG) //)
60  FORMAT (6F18.7)
    REWIND 1
    WRITE (1) (CMATRX(I,1),CMATRX(I,IB),CMATRX(I,IC),CMATRX(I,IO),
1      CMATRX(I,IM),CR(I),I=1,NXNY)
    RETURN
    END

```

```

/*
//GAUS1 EXEC FORTRAN(MAP)
SUBROUTINE GAUS1 (NX,NY,CMATRX,CP,H,IBC,EFAR,HP,NA,NB,
1      HCONV,ITER,IDEBUG,CUMTIM,DELTAT,JAGAIN,ICOUNT)
DIMENSION CMATRX(NA,NB),CR(1),H(1),IPC(1),EFAR(1),HP(1)
REAL*8 DH,DCR
C-----THE FOLLOWING EQUATION IS SOLVED
CC      AN*H'(IJ)-AN*H'(IJ-NX)-A*H'(IJ-1)-AE*H'(IJ+1)-AS*H'(IJ+NX) = -R(IJ)
CC      IN WHICH R IS A RESIDUAL, AND H' IS A CORRECTION VECTOR, H'=H(T+DT)-H(K)
CC
CC
CC      (IJ-NX)
CC
CC      THE SUBSCRIPT NOTATION (IJ-1) (IJ) (IJ+1)
CC
CC      (IJ+NX)
CC
CALL RSOLVE (CMATRX,NA,NB,CR)
NXNY = NX*NY
ITER = 0
NT = 0
C
DO 5 I=1,NX
DO 5 J=1,NY
IJ = NX*(J-1)+I
NT = NT+1
IF (ABS(CR(NT)).GT.HCONV) ITER = 1
DH = H(IJ)
IF (IBC(IJ).GE.3) CR(NT)=0.0
DCR = CR(NT)
DH = DH + DCR
H(IJ) = DH
5  CONTINUE
C

```

```

IF (ITER.EQ.0) GO TO 10
IF (DEBUG.EQ.1) CALL MAP (H,4HH ,2.,NX,NY,CUMTIM)
GO TO 20
10 CALL MAP (H,4HH ,2.,NX,NY,CUMTIM)
JCOUNT = JCOUNT + 1
IF (JCOUNT.EQ.1)
1 WRITE (6,11) JAGAIN,JCOUNT
IF (JCOUNT.EQ.2)
1 WRITE (6,12) JAGAIN,JCOUNT
IF (JCOUNT.EQ.3)
1 WRITE (6,13) JAGAIN,JCOUNT
IF (JCOUNT.GE.4)
1 WRITE (6,6) JAGAIN,JCOUNT
6 FORMAT (1H0,5X,22HNUMBER OF ITERATIONS =,I3,2X,5H FOR,I3,
1 13H-TH TIME STEP)
11 FORMAT (1H0,5X,22HNUMBER OF ITERATIONS =,I3,2X,5H FOR,I3,
1 12HST TIME STEP)
12 FORMAT (1H0,5X,22HNUMBER OF ITERATIONS =,I3,2X,5H FOR,I3,
1 12HND TIME STEP)
13 FORMAT (1H0,5X,22HNUMBER OF ITERATIONS =,I3,2X,5H FOR,I3,
1 12HRD TIME STEP)
20 CONTINUE
RETURN
END

```

```

/*BSOLVE EXEC FORTRAN(MAP)
SUBROUTINE BSOLVE (C,N,M,V)
C THIS SUBROUTINE SOLVES THE MATRIX BY GAUSS ELIMINATION.
DIMENSION C(N,M), V(N)
LR=(M-1)/2
DO 2 L=1,LR
IM=LR-L+1
DO 2 I=1,IM
DO 1 J=2,M
1 C(L,J-1)=C(L,J)
KN=N-L
KM=M-1
2 C(KN+1,KM+1)=0.0
LR=LR+1
IM=N-1
DO 10 I=1,IM
NPIV=I
LS=I+1
DO 3 L=LS,LR
IF (ABS(C(L,I)).GT.ABS(C(NPIV,I))) NPIV=L
3 CONTINUE
IF (NPIV.LE.I) GO TO 6
4 DO 5 J=1,M
TEMP=C(I,J)
C(I,J)=C(NPIV,J)
5 C(NPIV,J)=TEMP
TEMP=V(I)
V(I)=V(NPIV)
V(NPIV)=TEMP
6 V(I)=V(I)/C(I,1)
DO 7 J=2,M
7 C(I,J)=C(I,J)/C(I,1)
DO 9 L=LS,LR
TEMP=C(L,1)
V(L)=V(L)-TEMP*V(I)
DO 8 J=2,M
8 C(L,J-1)=C(L,J)-TEMP*C(I,J)
9 C(L,M)=0.0
IF (LR.LT.N) LR=LR+1
10 CONTINUE
V(N)=V(N)/C(N,1)
JM=2
DO 12 I=1,IM
L=N-I
DO 11 J=2,JM
KM=L+J
11 V(L)=V(L)-C(L,J)*V(KM-1)
IF (JM.LT.M) JM=JM+1
12 CONTINUE
RETURN
END

```

```

N 1
N 2
N 3
N 4
N 5
N 6
N 7
N 8
N 9
N 10
N 11
N 12
N 13
N 14
N 15
N 16
N 17
N 18
N 19
N 20
N 21
N 22
N 23
N 24
N 25
N 26
N 27
N 28
N 29
N 30
N 31
N 32
N 33
N 34
N 35
N 36
N 37
N 38
N 39
N 40
N 41
N 42
N 43
N 44
N 45
N 46
N 47
N 48
N 49

```

```

/*
//RESIDW EXEC FORTRAN(MAP)
SUBROUTINE RESIDW (NX, NY, RHS, H, IBC, EFAR, JAGAIN, IDEBUG, DELTAT,
1 AN, AS, AF, AW, AO, R, HP, CUMTIM)
1 DIMENSION RHS(1), H(1), IBC(1), EFAR(1), AN(1), AS(1), AE(1), AO(1), R(1),
1 AW(1), HP(1)
1 REAL*8 DH1, DHIC, DHIB, DHID, DHIM, DHPIM, DDT, DEFAR, DRHS, DAN, DAS,
1 CAE, DAW, DAO, DR, DE
DO 10 I=1, NX
DO 11 J=1, NY
IJ = NX*(J-1)+I
IF (IBC(IJ).GE.3) GO TO 5
DAN = 0.0
DAW = 0.0
DAS = 0.0
DAE = 0.0
DH1 = 0.0
DHIC = 0.0
DHIB = 0.0
DHID = 0.0
DHIM = H(IJ)
IF (I.EQ.1) GO TO 11
DH1 = H(IJ-1)
DH1 = DHIM - DH1
11 IF (J.EQ.NY) GO TO 12
DHIC = H(IJ+NX)
DHIC = DHIM - DHIC
12 IF (J.EQ.1) GO TO 13
DHIB = H(IJ-NX)
DHIB = DHIM - DHIB
13 IF (I.EQ.NX) GO TO 14
DHID = H(IJ+1)
DHID = DHIM - DHID
14 DHPIM = HP(IJ)
DDT = DELTAT
DEFAR = EFAR(IJ)
DRHS = RHS(IJ)
DRHS = DRHS*43560.
DE = (DEFAR/DDT)*(DHIM - DHPIM)
DRHS = DRHS + DE
DAN = AN(IJ)
DAS = AS(IJ)
DAE = AF(IJ)
DAW = AW(IJ)
DR = DAW*DH1 + DAS*DHIC + DAN*DHIB + DAE*DHID + DRHS
R(IJ) = -DR
IF (EFAR(IJ).EQ.0.0) R(IJ) = 0.0
IF (EFAR(IJ).EQ.0.0) AO(IJ) = 1.0
GO TO 10
5 AW(IJ) = 0.0
AS(IJ) = 0.0
AN(IJ) = 0.0
AE(IJ) = 0.0
AO(IJ) = 1.0
R(IJ) = 0.0
10 CONTINUE
IF (IDBUG.NE.1) GO TO 20
CALL MAP (AO, 4HAO , 2., NX, NY, CUMTIM)
CALL MAP (AW, 4HAW , 2., NX, NY, CUMTIM)
CALL MAP (AS, 4HAS , 2., NX, NY, CUMTIM)
CALL MAP (AN, 4HAN , 2., NX, NY, CUMTIM)
CALL MAP (AS, 4HAS , 2., NX, NY, CUMTIM)
CALL MAP (R, 4HR , 2., NX, NY, CUMTIM)
20 CONTINUE
RETURN
END

```

```

/*
//TRANSW EXEC FORTRAN(MAP)
SUBROUTINE TRANSW (RHS,EFAR,HP,Z,CE,CS,IBC,NX,NY,DELTAT,AN,AS,
1 AE,AW,AC)
DIMENSION RHS(1),FFAR(1),HP(1),Z(1),CE(1),CS(1),IBC(1),AW(1),
1 AE(1),AN(1),AS(1),AO(1)
REAL*8 DAW,DAE,DAN,DAS,DAO,DE,DEFAR,DDT
DO 100 I=1,NX
DO 100 J=1,NY
IJ = NX*(J-1)+I
DAW = 0.0
DAE = 0.0
DAN = 0.0
DAS = 0.0
AW(IJ) = 0.0
AN(IJ) = 0.0
AE(IJ) = 0.0
AS(IJ) = 0.0
IF (I.EQ.1) GO TO 3
1 CCW = CF(IJ-1)
AW(IJ) = CCW*(AMAX1(HP(IJ),HP(IJ-1))-AMAX1(Z(IJ),
3 Z(IJ-1)))
DAW = AW(IJ)
IF (I.EQ.NX) GO TO 4
1 CCE = CE(IJ)
AE(IJ) = CCE*(AMAX1(HP(IJ),HP(IJ+1))-AMAX1(Z(IJ),
4 Z(IJ+1)))
DAE = AE(IJ)
IF (J.EQ.1) GO TO 5
1 CCN = CS(IJ-NX)
AN(IJ) = CCN*(AMAX1(HP(IJ),HP(IJ-NX))-AMAX1(Z(IJ),
5 Z(IJ-NX)))
DAN = AN(IJ)
IF (J.EQ.NY) GO TO 6
1 CCS = CS(IJ)
AS(IJ) = CCS*(AMAX1(HP(IJ),HP(IJ+NX))-AMAX1(Z(IJ),
6 Z(IJ+NX)))
DAS = AS(IJ)
DDT = DELTAT
DEFAR = EFAR(IJ)
DE = DEFAR/DDT
DAO = DAW + DAN + DAS + DAE + DE
AO(IJ) = DAO
C-----Q IS OVERREAD BY RHS
RHS(IJ) = -RHS(IJ)
100 CONTINUE
RETURN
END

```

```

/*
//LSOR EXEC FORTRAN(MAP)
SUBROUTINE LSOR (AN,AS,AF,AW,AC,HST,R,A,B,C,D,NX,NY,OMEGA,IDEBUG,
1 CUMTIM,H,IBC,EFAR,HP,HCONV,ITER,DELTA,T,JAGAIN,
2 JCOUNT)
1 DIMENSION AN(1),AS(1),A(1),AW(1),AD(1),HST(1),A(1),B(1),C(1),
1 D(1),H(1),IBC(1),EFAR(1),HP(1),R(1)
REAL*8 DH,DHST
TOL = 0.0001
TOL = 0.01
TOL = 0.001
TOL=0.00001
NXNY = NX*NY
LS = 0
DO 1 K=1,NXNY
1 HST(K) = 0.0
2 ITER = 0
LS = LS + 1
DO 8 J=1,NY
DO 5 I=1,NX
HSTE = 0.0
HSTW = 0.0
HSTS = 0.0
HSTN = 0.0
IJ = NX*(J-1) + I
IF (J.NE.1) HSTJ = HST(IJ-NX)
IF (J.NE.NY) HSTS = HST(IJ+NX)
IF (I.NE.1) HSTW = HST(IJ-1)
IF (I.NE.NX) HSTE = HST(IJ+1)
A(I) = AN(IJ)
B(I) = -AN(IJ)
C(I) = AF(IJ)
5 D(I) = OMEGA *(-R(IJ)-AN(IJ)*HSTN - AS(IJ)*HSTS)
1 + (OMEGA-1.0)*(AO(IJ)*HST(IJ)-AE(IJ)*HSTE-AW(IJ)
2 *HSTW)
CALL THOMAS (D,A,R,C,D,NX)
C-----THE VECTOR D NOW CONTAINS THE NEW HST VALUES.
DO 55 I=1,NX
IJ = NX*(J-1) + I
IF (ABS(D(I)-HST(IJ)).GT.TOL) ITER = 1
HST(IJ) = D(I)
55 CONTINUE
8 IF (ITER.EQ.1) GO TO 2
ITER = 0
DO 500 IJ=1,NXNY
IF (ABS(HST(IJ)).GT.HCONV) ITER = 1
DH = H(IJ)
IF (IBC(IJ).GE.3) HST(IJ) = 0.0
DHST = HST(IJ)
DH = DH + DHST
H(IJ) = DH
IF (EFAR(IJ).EQ.0.0) H(IJ) = 0.0
500 CONTINUE
PRINT 9,LS
9 FORMAT (10X,15HLSOR ITERATIONS,I3)
IF (ITER.EQ.0) GO TO 10
IF (IDEBUG.EQ.1) CALL MAP (H,4HH ,2.,NX,NY,CUMTIM)
GO TO 20
10 CALL MAP (H,4HH ,2.,NX,NY,CUMTIM)
JCOUNT = JCOUNT + 1
IF (JCOUNT.EQ.1)
1 WRITE (6,11) JAGAIN,JCOUNT
IF (JCOUNT.EQ.2)
1 WRITE (6,12) JAGAIN,JCOUNT
IF (JCOUNT.EQ.3)
1 WRITE (6,13) JAGAIN,JCOUNT
IF (JCOUNT.GE.4)
1 WRITE (6,6) JAGAIN,JCOUNT
6 FORMAT (1H0,5X,22HNUMBER OF ITERATIONS =,I3,2X,5H FOR,I3,
1 12H-TIME STEP)
11 FORMAT (1H0,5X,22HNUMBER OF ITERATIONS =,I3,2X,5H FOR,I3,
1 12HST TIME STEP)
12 FORMAT (1H0,5X,22HNUMBER OF ITERATIONS =,I3,2X,5H FOR,I3,
1 12HND TIME STEP)
13 FORMAT (1H0,5X,22HNUMBER OF ITERATIONS =,I3,2X,5H FOR,I3,
1 12HND TIME STEP)
20 CONTINUE
RETURN
END

```



```

/*
//THOMAS EXEC FORTRAN(MAP)
SUBROUTINE THOMAS(U,A,B,C,D,NN)
DIMENSION A(NN),B(NN),C(NN),D(NN),U(NN),W(200),G(200)
C-----THIS SOLVES A TRI-DIAGONAL MATRIX BY THE THOMAS ALGORITHM.
C
C
C      TRI-DIAGONAL FORM IS
C      A*U(I-1) + B*U(I) + C*U(I+1) = D
C
W(1) = B(1)
G(1) = D(1)/W(1)
DO 60 I=2,NN
W(I) = B(I) - (A(I)*C(I-1))/W(I-1)
G(I) = (D(I)-A(I)*G(I-1))/W(I)
60 CONTINUE
U(NN) = G(NN)
DO 70 I=2,NN
J = NN - I + 1
U(J) = G(J) - (C(J)*U(J+1))/W(J)
70 CONTINUE
RETURN
END

```

```

/*
//BALCOM EXEC FORTRAN(MAP)
SUBROUTINE BALCOM (NX,NY,H,HP,EFAR,IBC,Z,G,CE,CS,DELTAT,
1 CUMTIM,CUMMB,SQVOL,ICOUNT)
COMMON/STOR/TSTOR,BIN,BOUT
DIMENSION H(1),HP(1),EFAR(1),IBC(1),Z(1),G(1),CE(1),CS(1)
C-----THIS SUBROUTINE COMPUTES THE MATERIAL BALANCE ERROR
C
C
C      GLOSSARY (OUTFLOW + ,INFLOW - )
C
C
C      QRVV      RIVER OR LAKE (OUTFLOW OF AQUIFER OR INFLOW OF AQUIFER)
C                (INFLOW)
C
C      BINFL     BOUNDARY INFLOW
C      BOUTFL    BOUNDARY OUTFLOW
C      PTSTOR    TOTAL STORAGE (PREVIOUS TIME LEVEL)
C      TSTOR     TOTAL STORAGE (NEW TIME LEVEL)
C      CSTOR     CHANGE IN STORAGE DURING DELTA-T
C      SQVOL     TOTAL WATER APPLIED (NET WITHDRAWAL)
C      SMB       MATERIAL BALANCE
C      CUMMB     CUMULATIVE MATERIAL BALANCE ERROR
C      CUMPC     CUMULATIVE MATERIAL BALANCE ERROR IN PERCENTAGE
C
CALL BINFLO (QRV, BINFL, BOUTFL, H, HP, CE, CS, IBC, Z, NX, NY)
PTSTOR = TSTOR
TSTOR = 0.0
SMB = 0.0
C
DO 1 I=1,NX
DO 1 J=1,NY
IJ = NX*(J-1) + I
IF (IBC(IJ).GE.3) GO TO 1
IF (ICOUNT.EQ.0)
1 PTSTOR = PTSTOR + EFAR(IJ)*(HP(IJ)-Z(IJ))
TSTOR = TSTOR + EFAR(IJ)*(H(IJ)-Z(IJ))
1 CONTINUE
C
C-----ALL VALUES CONVERTED INTO ACRE-FT FOR DELTAT PERIOD
IF (ICOUNT.EQ.0) PTSTOR = PTSTOR/43560.
BIN = -BIN * DELTAT
BOUT = -BOUT * DELTAT
SQVOL = -SQVOL * DELTAT
TSTOR = TSTOR/43560.
CSTOR = TSTOR - PTSTOR
BOUTFL = BOUTFL * DELTAT /43560.
BOUTFL = BOUTFL + BOUT
QRV = QRV * DELTAT /43560.
BINFL = BINFL * DELTAT /43560.
BINFL = BINFL + BIN
SMB = SMB + QRV + BINFL + BOUTFL + CSTOR + SQVOL
CUMMB = CUMMB + SMB
CUMPC = (CUMMB/TSTOR) * 100.0
C
WRITE (6,11) CUMTIM,DELTAT
WRITE (6,12) QRV
WRITE (6,13) BINFL
WRITE (6,14) BOUTFL
WRITE (6,15) SQVOL
WRITE (6,16) PTSTOR

```

```

WRITE (6,17) TSTOR
WRITE (6,18) CSTOR
WRITE (6,19) SMR
WRITE (6,20) CUMPC
11  FORMAT (1H0,29X,31HMATERIAL BALANCE AT THE END OF ,F6.1,18H DAYS(T
    TIME STEP = ,F5.1,7H DAYS)/30X,66H=====)
12  FORMAT (1H0,15X,72HFLOW FROM RIVER OR LAKE INTO(-)OR OUT OF(+) AQU
    IFER(AC-FT PER TIME STEP),3X,F12.2)
14  FORMAT (1H0,15X,57HFLOW OUT OF AQUIFER ALONG BOUNDARIES(AC-FT PER
    TIME STEP),18X,F12.2)
13  FORMAT (1H0,15X,55HFLOW INTO AQUIFER ALONG BOUNDARIES(AC-FT PER TI
    ME STEP),20X,F12.2)
15  FORMAT (1H0,15X,68HNET WITHDRAWAL(PUMPING,PRECIPITATION,PHREATOPHY
    ITE LOSS,LEAKAGE,ETC.),7X,F12.2)
16  FORMAT (1H0,15X,45HTOTAL STORAGE UP TO PREVIOUS TIME STEP(AC-FT),
    130X,F12.2)
17  FORMAT (1H0,15X,32HTOTAL STORAGE UP TO DATE (AC-FT),43X,F12.2)
18  FORMAT (1H0,15X,32HCHANGE IN STORAGE(AC-FT PER TIME STEP),37X,
    F12.2/91X,12H=====)
19  FORMAT (1H0,15X,24HMATERIAL BALANCE (AC-FT),51X,F12.2)
20  FORMAT (1H0,15X,26HCUMULATIVE ERROR (PERCENT),49X,F12.2,1H%)
    DO 8 I=1,NX
      DO 8 J=1,NY
        IJ = NX*(J-1)+I
        HP(IJ) = H(IJ)
8    CONTINUE
    RETURN
    END

```

```

/*
//BNDFLO EXEC FORTRAN(MAP)
SUBROUTINE 3BNDFLO (QRIV,BINFL,BOUTFL,H,HP,CF,CS,IBC,Z,NX,NY)
DIMENSION H(1),HP(1),CF(1),CS(1),IBC(1),Z(1)
C
QRIV = 0.0
BINFL = 0.0
BOUTFL = 0.0
C
DO 100 I=1,NX
  DO 100 J=1,NY
    IJ = NX*(J-1) + I
    IF (IBC(IJ).LT.3) GO TO 100
      QW = 0.0
      QE = 0.0
      QN = 0.0
      QS = 0.0
      AW = 0.0
      AE = 0.0
      AN = 0.0
      AS = 0.0
    HIJ = HP(IJ)
    IF (I.EQ.1) GO TO 3
      CCW = CF(IJ-1)
      AW = CCW*(AMAX1(HP(IJ),HP(IJ-1))-AMAX1(Z(IJ),Z(IJ-1)))
      QW = AW*(H(IJ)-H(IJ-1))
3    IF (I.EQ.NX) GO TO 4
      CCF = CF(IJ)
      AE = CCF*(AMAX1(HP(IJ),HP(IJ+1))-AMAX1(Z(IJ),Z(IJ+1)))
      QE = AE*(H(IJ)-H(IJ+1))
4    IF (J.EQ.1) GO TO 5
      CCN = CS(IJ-NX)
      AN = CCN*(AMAX1(HIJ,HP(IJ-NX))-AMAX1(Z(IJ),Z(IJ-NX)))
      QN = AN*(H(IJ)-H(IJ-NX))
5    IF (J.EQ.NY) GO TO 6
      CCS = CS(IJ)
      AS = CCS*(AMAX1(HIJ,HP(IJ+NX))-AMAX1(Z(IJ),Z(IJ+NX)))
      QS = AS*(H(IJ)-H(IJ+NX))
6    IF (IBC(IJ).EQ.4) GO TO 7
      IF (QE.GT.0.0) BINFL = BINFL + QE
      IF (QN.GT.0.0) BINFL = BINFL + QN
      IF (QW.GT.0.0) BINFL = BINFL + QW
      IF (QS.GT.0.0) BINFL = BINFL + QS
      IF (QW.LT.0.0) BOUTFL = BOUTFL + QW
      IF (QE.LT.0.0) BOUTFL = BOUTFL + QE
      IF (QN.LT.0.0) BOUTFL = BOUTFL + QN
      IF (QS.LT.0.0) BOUTFL = BOUTFL + QS
    GO TO 100
7    QRIV = QRIV -(QW + QE + QN + QS)

```

100 CONTINUE  
C

HINFL = -HINFL  
BOUTFL = -BOUTFL  
RETURN  
END

/\*  
//MAP EXEC FORTRAN(MAP)  
SUBROUTINE MAP (ARPA, NAME, FORM, NX, NY, DAYS)  
DIMENSION ARRAY(2), LABEL(20)

C  
DATA KCS /'CS ' //, KCE /'CE ' //, KH /'H ' //, KPPT /'PPT ' //,  
1 KPIW /'PIW ' //, KFERE /'FERE ' //, KLEA /'LEAK ' //, KSFE /'SFE ' //,  
2 KQ3C /'Q3C ' //, KZ /'Z ' //, KG /'G ' //, KFKX /'FKX ' //,  
3 KEKY /'EKY ' //, KDX /'DX ' //, KDY /'DY ' //, KPHI /'PHI ' //,  
4 KEFA /'EFA ' //, KHP /'HP ' //

C  
LMAX = 20  
IF (FORM.EQ.2) LMAX = 10  
N1 = 1  
LENGTH = NX  
IF (NAME.EQ.'KCE ') PRINT 10, DAYS  
IF (NAME.EQ.'KCS ') PRINT 20, DAYS  
IF (NAME.EQ.'KPPT ') PRINT 30, DAYS  
IF (NAME.EQ.'KPIW ') PRINT 40, DAYS  
IF (NAME.EQ.'KFERE ') PRINT 50, DAYS  
IF (NAME.EQ.'KLEA ') PRINT 60, DAYS  
IF (NAME.EQ.'KSFE ') PRINT 70, DAYS  
IF (NAME.EQ.'KQ3C ') PRINT 80, DAYS  
IF (NAME.EQ.'KFKX ') PRINT 90, DAYS  
IF (NAME.EQ.'KEKY ') PRINT 110, DAYS  
IF (NAME.EQ.'KDX ') PRINT 120, DAYS  
IF (NAME.EQ.'KDY ') PRINT 130, DAYS  
IF (NAME.EQ.'KPHI ') PRINT 140, DAYS  
IF (NAME.EQ.'KG ') PRINT 150, DAYS  
IF (NAME.EQ.'KZ ') PRINT 150, DAYS  
IF (NAME.EQ.'KH ') PRINT 170, DAYS  
IF (NAME.EQ.'KHP ') PRINT 190, DAYS  
IF (NAME.EQ.'KEFA ') PRINT 190, DAYS

10 FORMAT (1H0/40X, 21HCF MAP ,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
20 FORMAT (1H0/40X, 21HCS MAP ,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
30 FORMAT (1H0/40X, 21HPRECIPIATION MAP ,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
40 FORMAT (1H0/40X, 21HPIPING RATE MAP ,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
50 FORMAT (1H0/40X, 21HPOFAT. WATER MAP ,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
60 FORMAT (1H0/40X, 21HWATER LEAKING MAP ,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
70 FORMAT (1H0/40X, 21HWATER SEEPING MAP ,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
80 FORMAT (1H0/40X, 21HINFLOW (OUTFLOW) MAP,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
90 FORMAT (1H0/40X, 21HHYDR0. CONDOC.-X MAP,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
110 FORMAT (1H0/40X, 21HHYDR0. CONDOC.-Y MAP,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
120 FORMAT (1H0/40X, 21HX-INCREMENT MAP ,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
130 FORMAT (1H0/40X, 21HY-INCREMENT MAP ,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
140 FORMAT (1H0/40X, 21HEFF. POSISITY MAP ,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
150 FORMAT (1H0/40X, 21HGROUND FLEV. MAP ,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
160 FORMAT (1H0/40X, 21HSPRING ELEV. MAP ,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
170 FORMAT (1H1/40X, 21HWATER-TABLE MAP ,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
180 FORMAT (1H0/40X, 21HDEFINITS W.T. MAP ,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)  
190 FORMAT (1H0/40X, 21HEFFECTIVE AREA MAP ,14X,7HTIME = ,  
1 F10.4, 2X, 4HDAYS/)

2 CONTINUE  
IF (LENGTH.GT.LMAX) LENGTH = LMAX  
DO 3 I=1, LENGTH

```

3 LABEL(I) = N1-1+I
IF (FORM.EQ.1) PRINT 11,(LABEL(I),I=1,LENGTH)
IF (FORM.EQ.2) PRINT 12,(LABEL(I),I=1,LENGTH)
IF (FORM.EQ.3) PRINT 11,(LABEL(I),I=1,LENGTH)
11 FORMAT (1X,1X,20I6)
12 FORMAT (1X,10,9I12)
N3 = N1 - NX
DO 100 J=1,NY
N3 = N3 + NX
N4 = N3+LENGTH-1
IF (FORM.EQ.1) PRINT 101,J,(ARRAY(IJ),IJ=N3,N4)
IF (FORM.EQ.2) PRINT 102,J,(ARRAY(IJ),IJ=N3,N4)
IF (FORM.EQ.3) PRINT 103,J,(ARRAY(IJ),IJ=N3,N4)
101 FORMAT (1X,I2,20F6.0)
102 FORMAT (1X,I2,10F12.5)
103 FORMAT (1X,I2,2X,20F6.3)
100 CONTINUE
LENGTH = NX - N1 - LENGTH + 1
IF (LENGTH.LE.0) GO TO 99
N1 = N1 + LMAX
999 FORMAT (1X,///)
GO TO 2
99 RETURN
END

```

```

/*
//SYSA92 ACCESS SDSPHS
// EXEC LNKEED(MAP)
    PHASE PHASECCM,ROOT
    INCLUDE MAIN,L
    INCLUDE OLAY1,L
    INCLUDE SINGLE,L
    INCLUDE ARRAYS,L
    INCLUDE VALUES,L
    INCLUDE COEF,L
    INCLUDE DISTF,L
    INCLUDE WFIX,L
    INCLUDE BALCCM,L
    INCLUDE RNDFLD,L
    INCLUDE MAP,L
    PHASE PHASEGAU,*
    INCLUDE TRANSF,L
    INCLUDE RESIDG,L
    INCLUDE GAUS1,L
    INCLUDE BSOLVE,L
    PHASE PHASEFAT,PHASEGAU
    INCLUDE TRANSW,L
    INCLUDE RESIDW,L
    INCLUDE LSN2,L
    INCLUDE THOMAS,L

```

```

/*
// EXEC
CMNT
NCOM          44
OMEGA         1.5
IWAT
IGAU
HCONV         0.01
DEBUG
DELTAT        91.25
NX            15
NY            26
ENDS
CMNT
CMNT
CMNT
CMNT
CMNT
H              4000.
FREA
QBC
SEFP
LEAK
Z              3900.
FKX            40.
FKY            40.
PHI            0.15
G              4100.
PPT
DY            1.

```





